

# Main Street Recreation Complex Feasibility Study Town of Lynnfield, MA

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Gale JN 715630

### MAIN STREET RECREATION PARK FEASIBILITY STUDY LYNNFIELD, MA

#### SECTION 1.0 - BACKGROUND AND FEASIBILITY STUDY OBJECTIVES

Gale Associates, Inc. (Gale) was engaged in September of 2012, to assist the Town of Lynnfield, Massachusetts (the Town), to complete a limited feasibility study for the development of an athletic and recreation complex at a group of twelve (12) undeveloped parcels of land located in the Town. The parcels are currently owned by the Lynnfield Center Water District (LWD) and are located to the northwest of Main Street in the Town and to the south of Elm Street in the Town of North Reading.

The parcels are located in the "Single Residence D" zone, based on the Lynnfield Zoning map. The parcels do not contain frontage on any right of way in the Town and are currently accessed through LWD property off of Elm Street in North Reading. (Refer to the enclosed Existing Conditions Plans for Book and Map information for the parcels.) The parcels will be referred to as a single parcel, the "Main Street Parcel," throughout this report. The combined parcel total approximately 98.5 acres in size. The parcel is extensively forested and contains a network of cart paths and trails throughout the site. An abandoned railroad easement extends across the northern portion of the property, running east to west. The northern limits of the parcel are bordered by the Ipswich River, while Wills Brook flows across the southern portion. Property to the east consists of undeveloped lands and single-family dwellings along Main Street, while an industrial building owned by Bostik exists to the northeast. Property to the west is primarily town-owned, undeveloped areas and property to the south and southeast is occupied by the Sagamore Springs Golf Club.

The feasibility study is intended to coincide with the development of a Recreational and Athletic Field Master Plan for the Town and to determine the viability of developing a Recreation Complex at the Main Street Parcel. Based on the results of the Master Plan, the Town has a deficit of approximately five to six (5-6) multipurpose athletic fields and a significant perceived need (by Town's citizens') for additional recreational programming space. The intent of this feasibility study is to determine what recreational needs can be met through development of the Main Street Parcel.

The process used to complete the feasibility study focused on a specific set of tasks, which are summarized as follows.

1. To perform a background investigation, including property line survey, wetland delineation and geotechnical test pits, to determine the geotechnical, topographical and resource area constraints that may impact the development potential of the site.

- 2. To determine how the existing parcel may be developed and configured to best meet the athletic facility needs of the Town, specifically focusing on the priorities resulting from the Recreational and Athletic Field Master Plan.
- 3. To prepare a set of schematic level plans for the Recreation Complex proposed at the Main Street Parcel.
- 4. To design and prepare permitting level plans and cost estimates for a roadway to the Recreation Complex, accessible from Main Street, through property owned by Bostik, for which an access agreement is being negotiated by the Town.

This report documents the prevailing site conditions, conceptual development layout, predesign cost estimates and the permitting requirements to allow the Town to determine if development of the Main Street Parcel is feasible to meet the Town's recreational needs based on the Recreational and Athletic Facility Master Plan.

#### SECTION 2.0 - EXISTING CONDITIONS AND BACKGROUND INVESTIGATION

In order to facilitate the planning of the proposed athletic facility development, Gale's survey consultant, Eaglebrook, LLC, performed a property line survey, incorporating topographic information provided by the Town. Gale's wetland consultant, LEC Environmental Consultants (LEC), performed environmental resource area delineations to locate rivers, streams, bordering vegetated wetlands and bordering land subject to flooding throughout the parcel. An Existing Conditions Plan was compiled for the parcel and contains property line data, topography and the locations of wetland flags. The Existing Conditions Plan is attached as Enclosure 1, and LEC's Resource Report is attached as Enclosure 4.

Gale completed multiple site visits to the parcel to evaluate the topography, groundcover, geology, accessibility, proximity of abutters and locations of existing structures.

2.1 Site Description. THE LWD property is bounded to the north by the Ipswich River, to the west by Conservation Commission land, to the south by the Sagamore Golf Club and to the east by land owned by Bostik. The property consists of twelve (12) parcels owned by the LWD and is approximately 98.5 acres in size. The property does not contain frontage on any right of way, but is located just north of Main Street and south of Elm Street. The property can be accessed from Elm Street, at the entrance to the existing LWD pumping station, and through the abandoned railroad easement, which runs through the property from east to west. Refer to the Existing Conditions Plan (Enclosure 1) for property line and topographic information.

2.1.1 Soils. Gale used two (2) methods to determine the soil profile characteristics throughout the limits of the parcel. The first method used was through the Natural Resources Conservation Service (NRCS), which provides soil classification mapping for a specific area. The second method was through deep hole test pits, which were conducted by the Lynnfield Department of Public Works (DPW) and observed by Gale. They were placed at six (6) locations throughout the parcel, to determine the soil profiles, parent materials, depth to groundwater and drainage characteristics of on-site subsurface soils.

Because of the significant size of the parcel, the NRCS Web Soil Survey identified several classifications of soils existing within the limits of the parcel. The following is a summary of the classifications that occurred most frequently within the parcel.

"255B - Windsor Loamy Sand, 3% - 8% slopes." The parent material for this classification of soil consists of loose sandy glaciofluvial deposits. Its soil profile consists of loamy sand and sand, with excessive draining capabilities and a depth to groundwater greater than 80".

"323C - Poquonock loamy sand, 8% - 15% slopes, very stony." The parent material for this classification of soil consists of loose sandy glaciofluvial deposits over dense loamy lodgment till. Its soil profile consists of loamy sand, loamy fine sand, and gravelly fine sandy loam, considered to be well drained and with a depth to groundwater of about 18" to 36".

"311B - Woodbridge fine sandy loam, 3% - 8% slopes, very stony." The parent material for this classification of soil consists of friable coarse-loamy eolian deposits over dense coarse-loamy lodgment till. Its soil profile consists of fine sandy loam and gravelly fine sandy loam, and is considered to be moderately well drained with a depth to groundwater of approximately 18" to 36".

"73A - Whitman loam, 0% - 3% slopes, extremely stony." The parent material for this classification of soil consists of friable coarse-loamy eolian deposits over dense coarse-loamy lodgment till. Its soil profile consists of loam, gravelly fine sandy loam and fine sandy loam with very poor drainage capabilities due to a restrictive fine loamy layer. Depth to groundwater is estimated at 12"-25".

Refer to the enclosed NRCS Web Soil Survey (Enclosure 2) for the soil mapping results of the entire parcel.

While the NRCS Web Soil Survey can be used to approximate soil classifications, Gale observed a series of test pits to provide a more accurate soil profile analysis at specific locations. Seven (7) test pits were excavated, sampled and a soil log completed for each. Refer to the enclosed Test Pit Logs (Enclosure 3) for the results. Because of extensive wetland resource areas in the parcel, the test pits were conducted in the main upland area, located at the northern portion of the parcel. Test pits were not performed along the proposed Access Drive due to challenging access and proximity to wetlands. We recommend that soil testing in this area be performed prior to developing detailed construction drawings. Refer to the Existing Conditions Plan (Enclosure 1) for the test pit locations and Test Pit Logs (Enclosure 3) for soil profile data. The following is a summary of the geotechnical test pit data.

The profile of the subsurface soils located within the northern portion of the LWD parcel generally includes a 6" to 10" layer of loam and organics, underlain by 24" to 52" of a combination of loamy sand, loamy fine sand, fine sand and medium sand, depending on the location of the test pit. While these classifications can vary in textural analysis, the drainage characteristics of these layers can be considered moderately to well draining. The parent materials for the majority of the test pits consisted of glaciofluvial deposits, with some lodgment till at the greatest depths of the test pits. The soils at the greatest depths of the test pits (84" to 102" depths) consisted mainly of sand or fine to medium sands, with high draining capabilities. Indicators of groundwater, such as redoximorphic features and/or standing water were not discovered in the test pits. Based on the information collected during the test pits, it appears that the upland areas of the parcel contain well-draining soils, with no observations that indicate an existence of a high groundwater table.

Additionally, there were no observations of ledge or rock outcroppings that would indicate excavation refusal. It can be concluded that the existing subsurface soil, in areas where test pits were excavated, lends itself well to recreational facility development. The sandy and often gravelly materials are suitable as a sub-base material for natural or synthetic turf due to their granular consistency, high infiltration and compaction capabilities and low percentage of silts and fines. Additionally, depths to groundwater are not anticipated to be of concern in the areas where test pits were conducted.

2.1.2 Environmental Resource Areas. Gale and our environmental subconsultant, LEC, performed an analysis of environmental resource areas throughout the parcel. The following is a summary of the findings. Refer to the Environmental Resource Report attached as Enclosure 4 for more information.

According to the 13th Edition of the Massachusetts Natural Heritage Atlas, the property is not located within an Estimated or Priority Habitat for rare species. According to MassGIS, there are no certified or potential vernal pools on the site.

A FEMA Flood Insurance Rate Map for the Town indicates that the site contains extensive areas mapped as Zone "A" Areas, which are subject to inundation by the 1% annual chance of flood (100-year flood zone). The upland portions of the property at mapped as Zone "X" Areas and are determined to be outside the 100-year floodplain.

The site is contained within a Zone II wellhead protection zone and the local Groundwater Protection District. The Zone II wellhead designation restricts the amount of impervious area that can be developed without recharge and proper treatment. Based on the Zoning Regulations in the Town, recreational uses are permitted within the Groundwater Protection District.

The LEC performed wetland delineation throughout the entire parcel to determine locations of environmental resource areas, such as rivers, streams, bordering vegetated wetlands, vernal pools and bordering land subject to flooding that could potentially impact feasibility of site development. For the results of the wetland delineation, please refer to Enclosure 4.

Based on the wetland delineation performed, there appears to be approximately 15,000 linear feet of flagged wetland resource areas. This results in a 100' buffer, restricting the limits of developable, upland area and requiring permitting through the Conservation Commission. Included in the wetland resource areas on site are the Ipswich River, which is protected by a 200' Riverfront Area Buffer, an extensive amount of Bordering Vegetated Wetlands (BVW), which are protected by a 100' wetland buffer, and the Wills Brook, which is protected by a 200' Riverfront Buffer due to its classification as a perennial stream. In addition to the buffers described above, the Town imposes a 25' No-Disturb Zone and 50' No-Build Zone to further protect the wetland resource areas. Any development within the buffer zones described above would require permitting through the Lynnfield Conservation Commission.

The following is a summary of the approximate totals of wetlands, buffers and upland areas at the Main Street parcel:

Limits	Area (Acres)
Entire Parcel	98.5 acres
Delineated Wetlands	47.8 acres
25' No-Disturb Buffer	9.8 acres
Area Between 25' and 100' buffer zone	15.9 acres

Given the areas provided in the table above, the following chart summarizes the limits of upland areas that can be considered feasible for development. The twenty-five foot (25') No-Disturb Zone is considered to be unusable, or would require substantial permitting. The upland area outside of all buffers and requiring the least amount of permitting totals approximately twenty-five (25) acres. The upland areas within the 100' buffer, but outside of the twenty-five foot (25') buffer, totals approximately 40.9 acres, and would require permitting for the work proposed within the buffer limits.

Limits	Area (Acres)
Uplands (outside of all wetlands and buffers)	25 acres
Uplands (within 100' buffer, but outside of 25'	40.9 acres
buffer)	

Gale prepared and submitted an Abbreviated Notice of Resource Area Delineation (ANRAD) to the Lynnfield Conservation Commission on behalf of the Town and attended a public hearing on December 11, 2012. The application is currently on hold until the Town determines if it will continue with the permitting process for any proposed development at the Main Street Parcel. Refer to Enclosure 5 for the completed ANRAD application.

**2.1.3 Topography.** Topography data for the Main Street Parcel was compiled from record drawings provided by the Town and was verified and supplemented via on-the-ground survey.

The topography of the site generally contains low-lying wetlands toward the property lines, with several drumlins and hills between ten and fifteen feet (10'-15') in peak height throughout the site. The slopes of the upland areas range throughout the site, between one and eight percent (1%-8%), with the greatest slopes occurring over the Bostik parcel located just to the north of Main Street. At this location, Main Street is the high point and the grade generally decreases at a nine percent (9%) slope toward the LWD property.

Several small hills and flat plains exist throughout the parcel, with the flattest areas located in the low-lying wetlands. At the northern portion of the site, slopes vary between one and five percent (1% and 5%), with the exception of the small hills and drumlins, which reach slopes between eight and twenty percent (8% and 20%). At the western portion of the site a large hill exists, with a toe slope and flat plain surrounding it, adjacent to the Wills Brook. The grades in this area generally slope from south, at the property line, to north where wetlands exist.

Refer to the Existing Conditions Plan (Enclosure 1) for topography information, including contours at a two-foot (2') interval. The approximate elevation at the southern property line is 90' - 100'. The approximate elevation at the northernmost property line is approximately 60' - 70'. However, several drumlins, hills and lowlying wetlands exist between the property lines.

For this potential development, a concern is the earthwork associated with the construction of a roadway to access the parcel from Main Street, where the existing topography is sloped at 9%± over the Bostik Parcel and the length of roadway would exceed 1800'. Additionally, because of the close proximity of wetlands in this area, significant retaining walls will be required to reduce development impact on wetlands and meet existing grades. In the northern and western portions of the site, earthwork will consist mainly of excavation, in which case a significant portion of the materials could likely be used in the construction of the access drive. Further detail on the impacts of the proposed development will be described in later sections.

2.1.4 Utilities. Due to the remote location of the Main Street parcel and a lack of right of way frontage, existing or accessible utilities within the site are limited. The only utilities existing within the parcel are a series of drilled wells maintained by the LWD, which are scattered throughout the site and are not near the proposed complex development. Potable water is supplied to the adjacent Main Street residences, Bostik and Sagamore Country Club properties via the LWD service supplied through the Massachusetts Water Resource Authority (MWRA). There is currently no water service line to the site.

Electric service is provided to the adjacent streets and parcels through the Reading Municipal Light Department (RMLD). There is currently no service provided to the parcel from either Main Street or Elm Street. Installation of either electric service or a generator will be required upon development of the parcel. Proposed electric service provisions are discussed in Section 3.5 of this report.

No connections to either sanitary or storm drain systems within the Town currently exist from the parcel. Development of the parcel will likely require the installation of a septic system, as well as provisions for on site drainage mitigation, which may consist of a network of structures and piping, as well as detention chambers, ponds or other stormwater management practices. Proposed sanitary and storm drain systems are discussed in Sections 3.4 and 3.5.

Development of the recreation complex, including an access roadway, parking, an amenities building and lights, will require provisions for electric, telecommunications, gas, water, sanitary and storm drain services to the parcel, as none currently exist. The discussion of the proposed schematics will provided further detail on provisions for these services.

#### SECTION 3.0 - SCHEMATIC DESIGN - RECREATION COMPLEX

The schematic design for the proposed Recreation Complex is intended to coincide with the recreational needs, as determined by the Recreation and Athletic Field Master Plan. The Master Plan process included results of a Community Survey, feedback from members of the community during development of the Master Plan, as well as a conclusive Planning Program developed to meet the community's recreational needs. The intent of the Master Plan was to provide an evaluation of the current recreational facilities, determine the demands placed on these facilities and estimate the deficiencies that the Town is currently facing in regards to unmet recreational needs. Through use of community sensing sessions, a town-wide survey and several meetings with the Fields Committee, a planning program has been developed that is intended to address the unmet recreational needs within the Town. While assessing redevelopment of existing athletic facilities is included in the Master Plan, Gale has also been tasked with a feasibility study for the currently undeveloped Main Street Parcel.

Based on results of the field evaluation and needs assessment, there is currently an unresourced demand for athletic fields. The Master Plan provides details of the analysis, which concluded that the Town has a deficit of five to six (5-6) Multi-Purpose Rectangular (MPR) athletic fields. Additionally, sensing sessions and a community-wide survey provided feedback from Town residents that additional walking, running and multi-use trails are desired. Additionally, results indicated that there is a perceived need for additional lighted athletic fields, an ice-skating facility, additional field maintenance

procedures, an indoor recreation complex and a dog park. The intent of the Master Plan is to provide a planning program that accomplishes the recreational needs of the community. A combination of redevelopment at existing facilities and a proposed new development at the Main Street parcel is intended to better meet the existing recreational needs in the Town.

Gale developed a schematic design for the Main Street Recreation Complex as part of the Master Plan. The development can be broken down into three components or main areas: (1) Active Recreation (soccer, lacrosse, basketball, etc.), (2) Passive Recreation (walking, jogging, biking, dog-walking, etc.), and (3) Access Roadway. Because the majority of the area suitable for development is in the northern portion of the parcel, a significant portion of the development will be a roadway to access to the recreational development. The following is a discussion of each of the proposed development areas. Refer to Enclosure 6 for the schematic level concept plans for each alternative.

#### 3.1 Main Street Parcel - Active Recreation Components - Option 1

The active recreation complex is proposed at the northern portion of the parcel and will be or is to be accessible from the proposed roadway off Main Street. The complex is proposed to include MPR athletic fields, tennis and basketball courts, an ice-skating rink, a central amenities building, playground areas, multi-use paths and parking areas. The following is a discussion of each of these components.

Multi-Purpose Fields. The proposed layout provides for three (3) multi-purpose athletic fields, sized to accommodate Massachusetts Interscholastic Athletic Association (MIAA) events, including soccer, lacrosse and football, as well as a variety of youth level recreation programs. The layout provides for a minimum of ten-foot (10') safety zones surrounding the perimeter of each field. Under Option 1, one (1) of the multi-purpose fields is proposed as synthetic turf while the other two (2) fields are proposed as natural turf fields. While the decision of natural versus synthetic turf can be determined at a later date, the schematic design and results of the Master Plan for Option 1 assume that two (2) of the three (3) fields will be natural turf. A synthetic turf field can accommodate greater than 500 uses per year, while a well-maintained natural turf field can accommodate a maximum of 250 uses per year and still maintain an adequate playing surface.

The dimensions of the multi-purpose fields are as follows:

MPR Field 1 – Synthetic Turf:	210' x 360'
MPR Field 2 – Natural Turf:	210' x 345'
MPR Field 3 – Natural Turf:	210' x 330'

Hard Courts. A tennis court and basketball court are proposed to be developed within the Recreation Complex to provide a variety of active recreation options to community users. While tennis and basketball were not the leading results of the recreational needs survey, they are relatively inexpensive and provide a recreational use that meets the needs of a wide range of recreation users. Additionally, there are few tennis and basketball facilities in the Town and the majority of those facilities are reaching the end of their useful lives, including the tennis court facility at Newhall Park.

The courts are proposed in a north-south orientation, which is optimal for tennis and basketball use. A path is provided to the courts for pedestrian access from the parking lots and multi-use trails. Both courts are proposed with a ten-foot (10') vinyl-coated chain link fence and acrylic surfacing. Lights are not proposed at the tennis/basketball complex.

Ice Skating Rink. Based on results of the Master Plan Needs Assessment, an ice-skating or roller-hockey rink is desired within the Town. Currently, a Town owned and operated ice-skating facility does not exist within the Town. The proposed Recreation Complex includes a seasonal, outdoor skating/roller hockey rink sized adequately for public use at 85' x 175'.

Parking. The schematic includes parking to accommodate 289 parking stalls. Gale uses the following calculation to determine parking needs for athletic field and recreational facilities:

Calculating Parking for 1 Field:
40 players (2 teams) \* 2 \* 1.5 spectators \* 60% = 72 spaces

The Main Street Recreational Complex proposes three (3) multi-purpose fields, which will require approximately 216 spaces based on the above calculation. Additionally, an extra 73 spaces are proposed to accommodate users of the basketball, tennis, skating rink and trails. We feel that the proposed parking is adequate to accommodate users of the active recreation portion of the parcel.

Spectator Seating. The schematic layout provides a quantity of 186 seats at the synthetic turf field, plus an additional 93 seats at each of the natural turf athletic fields. The spectator seating is proposed as a forty-foot (40') long, five (5) row, aluminum seating system intended to be installed on a concrete pad. Two (2) units are proposed at the synthetic turf field and one (1) unit at the natural turf fields.

Amenities Building. An amenities building is proposed in a location central to the athletic fields, hard court spaces, playground and trails, and is intended to provide concessions, storage and restrooms. The building, as proposed, is 1,800 square feet and includes an overhead garage door and open storage area, six (6) female restroom facilities, three (3) male restroom facilities and a concessions facility appropriate for packaged goods and the use of small appliances only. The area surrounding the amenities building is proposed to include picnic tables, benches and queuing areas for spectators.

Playground. This schematic layout provides for two (2) age-appropriate play areas, one (1) designated for two to five (2-5) year olds, the other for six to twelve (6-12) year olds. The playgrounds are proposed to include pre-fabricated playground structures, based on National Playground Safety Institute standards, and include four-foot (4') vinyl-coated chain link fence, gates and seating areas for playground users.

Pedestrian Circulation. Bituminous concrete walkways are proposed to provide access to the facilities from each of the parking lots. Additionally, an eight-foot (8') wide walking path is proposed to loop around the facility and provide circulation to each facility, as well as connections to some of the existing paths and the abandoned railroad easement path. The walkways and trails within this portion of the recreation complex total approximately 6,000 linear feet, or 1.1 miles.

#### 3.2 Main Street Parcel - Active Recreation Components - Option 2

Because of the geometrical constraints of the property and the significant amount of wetlands minimizing the developable area, there are a limited number of development alternatives possible to meet the build-out needs of the parcel. Option 1 of the Main Street Parcel redevelopment is intended to be paired with Option 1 for the High School and Middle School redevelopment. Therefore, the needs of the planning program will determine the build-out for each strategy. Option 1 of the Main Street complex, assuming redevelopment of the High School and Middle School Option 1 strategies requires that an additional two to three (2-3) fields be provided to meet the planning program. Therefore, Option 1 of the Main Street complex proposes one (1) synthetic turf field and two (2) natural turf fields.

Option 2 of the Main Street complex is intended to be paired with Option 2 for the High School and Middle School, and will require an additional five (5) fields be provided through development of the Main Street Parcel. Therefore, under Option 2, two (2) of the multi-purpose fields are proposed to be synthetic turf and one (1) of the multi-purpose fields will be natural turf.

The Option 2 layout proposes identical program elements to Option 1, including three multi-purpose fields, a tennis court, basketball court, skating rink, amenities building, playgrounds and pathways. The main difference in Option 2, outside of the synthetic versus natural turf, is a central parking area and a relocated tennis and basketball complex.

Based on the redevelopment alternative(s chosen for the High School and Middle School, the planning program will determine what capacity of development will be required of an undeveloped parcel, such as the Main Street Parcel. Options 1 and 2 of the Main Street Recreation Complex provide alternatives that could accomplish the goals of the planning program at the Main Street Parcel.

#### 3.3 Main Street Parcel - Passive Recreation Components

Along the southern property line of the parcel lies an upland area, of which approximately seven (7) acres exists outside of all wetland and buffer zones. Because of the significant amount of trails existing along this portion of the property today, and because this area of the parcel is narrow and not ideal for the development of multi-purpose fields, passive recreation opportunities are proposed here.

The passive recreation area is accessed via a driveway that intersects the Main Street Recreation Complex's driveway. The driveway leads to a parking area sized to accommodate fifty (50) patrons. From the parking area, visitors have access to the corridor parkway, dog parks, gardening plots and trail network, all of which are discussed in additional detail below.

Multi-Use Trails. The most compelling need resulting from the community-wide Needs Assessment Survey was determined to be additional walking, biking, running, jogging and multi-purpose trails. Based on this need, the passive recreation area is proposed to include a series of trails, including a one (1) mile long cross country loop, marked walking paths and cross-country biking trails.

The main multi-use trail is proposed to begin in the open space of the corridor parkway, adjacent to the proposed parking lot. After a short distance, the trail will tie into an existing gravel path, which is proposed to be paved, and follow the path for approximately 800°. The trail continues along the western portion of the parcel, looping around an existing hill that provides approximately fifteen feet (15°) in elevation change. The trail is proposed, at twelve feet (12°) wide, to accommodate running, walking and biking in each direction and is intended to provide marked trail distances, as well as seating areas along the path. The trail is primarily flat, with a few areas of mild slopes. The path loops back around the passive recreation area and totals one (1) mile upon its return to the corridor parkway at the parking lot.

In addition to the multi-use trail in the southern portion of the parcel, a connector trail is proposed to be constructed across the wetlands to connect the southern and northern areas of the parcel. The trail is proposed as an eight-foot (8') wide gravel path for cross-country running, walking and biking, and as an access point between the active and passive recreation areas. The trail would require permitting to allow its construction through the wetlands, for a length of approximately 530'.

**Dog Park.** A dog park of just under one (1) acre in size is proposed within the Passive Recreation Area. The park is divided into two (2) areas, one (1) for large dogs and one (1) for small dogs. The park is enclosed with six-foot (6') perimeter fencing and includes landscaping trees, rocks and shrubs and is proposed to be surfaced with three-quarter inch (3/4") crushed stone. The park is accessible via the driveway and parking lot in the passive recreation area and also by the trail system connecting the active and passive recreation areas.

Gardening Plots. In an effort to provide recreational opportunities for all age groups and recreational interests, a community gardening plot is proposed. The proposed area is approximately 7,300 square feet and contains 48 plots of approximately 120 square feet each, as well as walking paths between them. The gardening area is enclosed with four-foot (4') chain link fencing.

Open Space / Picnic Areas. Through the center of the passive recreation area, a corridor parkway is proposed to provide some open space areas ideal for walking, sitting or having picnics. The treeline would be opened up to provide open space where benches, paved walkways and landscaping are proposed. The area of the park is just over an acre and is situated between the multi-use trail loop, providing access to the trail at several points in the park.

The passive recreation area is intended to provide a variety of recreation opportunities outside of the active recreation programming proposed in the northern portion of the complex. With adequate parking, lighting, multi-purpose trails and open space, the southern portion of the facility will accommodate those passive recreation needs ascertained from the community-wide survey.

#### 3.4 Grading and Drainage

The active recreation site will be primarily composed of natural and synthetic turf fields, hard court space, pedestrian walkways, parking lots and landscaping. As compared to the existing site, which contains varying topography ranging from one to thirty percent (1% - 30%) slopes, the proposed grade will be generally flat.

Natural turf fields are proposed to be crowned and sloped at a 1.5% – 2% slope, synthetic turf fields at 0.5%, and hard court spaces ranging between 0.5 and 1% percent slopes. The parking lots are proposed to be sloped between 1.5% and 2%. Other than the proposed drainage areas, including bio-retention and detention basins, the proposed limits of work will be generally flat.

The active and passive recreation sites will require a significant amount of earthwork to achieve the proposed grades for the athletic fields and parking lots, primarily consisting of excavation and earthwork cuts. There are several drumlins, as well as gradually sloping topography, located throughout the property where the recreation complex is proposed. Gale performed a cut/fill analysis to determine the extent of earthwork required to achieve proposed grades at the recreation areas, excluding the earthwork associated with the access roadway. The result of the analysis is a net cut of approximately 34,300 cubic yards. As will be discussed in a later section, an earthwork analysis on the access roadway was also completed, resulting in a net fill. Based on the results of the test pits and the NRCS soil information, the excavated materials will likely be suitable for use as common fill for the proposed roadway. Refer to Section 4 for additional information related to the cut/fill analysis of the proposed access roadway.

The drainage system for the recreation complex will include subsurface drainage for each of the multi-purpose fields, as well as bio-retention areas, subsurface detention chambers, detention ponds, conveyance structures and pipes to mitigate peak flows and meet State Stormwater Standards. The proposed synthetic turf fields will mainly drain vertically through the infill surface and the openly graded crushed stone drainage layer. Stormwater will infiltrate into the subgrade material beneath the stone base and provide recharge to groundwater. The stormwater that does not infiltrate will be transported by flat panel drains to perforated collector pipes located in stone trenches at the sidelines, further promoting groundwater recharge. Water from the collector pipes will be released at a controlled rate into the proposed drainage system. The synthetic turf fields provide a significant amount of groundwater recharge and peak flow attenuation, assisting to offset the potential stormwater impacts created by the proposed impervious areas.

The natural turf fields will consist of a sand-based, engineered root zone to allow stormwater to infiltrate and reach a four inch (4") drainage layer, consisting of crushed stone, beneath the fields. Stormwater that does not infiltrate will be transported through the proposed drainage system, to the proposed bio-retention and/or detention ponds, which will release stormwater to the wetlands at a controlled rate.

The larger parking facilities will facilitate drainage through subsurface detention chambers and bio-retention areas for treatment to meet Stormwater Standards for water quality and peak rate attenuations. Water quality and treatment of new impervious runoff will be achieved through vegetated filter strips, deep-sump hooded catch basin structures and bio-retention areas.

All runoff from the site will be contained, treated and released at a controlled rate to the existing wetlands to attenuate peak flows. All Massachusetts Stormwater Standards will be required to be met upon development of the parcel.

#### 3.5 Utilities

As described in a previous section, existing utilities accessible from within the parcel are limited. Provisions for electric, water, sanitary and gas services will be required upon development of the parcel. As the site development is essentially isolated within the northern portion of the parcel, utility services will be installed concurrently with the roadway construction.

#### 3.5.1 Electric

Gale consulted with Verne G. Norman Associates, Inc. (VGNA) to determine the requirements for electric service to the Recreation Complex for athletic field lighting, site lighting and service to the proposed amenities building. Refer to Enclosure 10 for information on the recommendations by VGNA. A summary of the required electrical service is provided below.

Electrical service would be provided by the Reading Electric Light Company. Single phase power is currently available on Main Street, at the location of the proposed complex entrance. Three phase power would not be readily available at the proposed facility entrance, but is located approximately 0.67 miles southwest of the site, on Main Street, adjacent to the Sagamore Golf Course. Musco Lighting has confirmed that single phase lighting will be sufficient to power the sports lighting, as the three phase power would be a substantial cost, given the existing service location. The anticipated electrical load includes 5.0 kilowatts (KW) for roadway lighting, 5.0 KW for parking lighting, 150 KW for sports lighting, and 25 KW for the concessions building, for a total electrical load of 185 KW. The site secondary electrical service would be 120/240 volt single phase, three wires, rated at 1000 amperes.

It is recommended that LED lighting for the roadway and parking lighting be considered to reduce the operating and maintenance costs.

#### 3.5.2 Water

Potable water service is not currently provided to the parcel. The proposed development would require water for irrigation and potable water service to the amenities building. The LWD operates a pumping station, adjacent to Elm Street in North Reading, which is located approximately one half (1/2) mile from the proposed development. Gale understands that the LWD owns a series of drilled wells located throughout portions of the LWD parcel. Water service to the proposed development will likely require drilled wells for irrigation and potable water, estimated to cost approximately \$14,000 - \$20,000 for each well and pump. The cost of construction will vary, depending on drilling depths and geology. The Town currently operates drilled wells at nearly every field location and effectively runs irrigation from

these wells. Maintenance of the irrigation system and wells will be the responsibility of the DPW.

#### 3.5.3 Sanitary Sewer

The Town does not currently have a town-wide sanitary sewer system. Therefore, a septic system compliant with Title 5 regulations will be required to treat wastewater from the amenities building. In the majority of areas where test pits were conducted, the separation from groundwater and classification of the subsurface and parent materials will likely be suitable for the installation of a traditional septic system, including a septic tank, gravity distribution system and soil absorption system (SAS) or leaching field. Prior to the design of the septic system, a series of test pits will be required to confirm material classifications and depth to groundwater.

#### 3.5.4 Natural Gas

Natural gas service is not currently provided to the site. Development of an amenities building will require gas service, unless the Town chooses to operate the building seasonally. Residents and businesses in the Town currently obtain natural gas service from National Grid. It is assumed that a natural gas main is currently installed on Main Street, as several adjacent residential homes and commercial buildings on Main Street are provided with natural gas service. The service will be required extended from Main Street, an approximate distance of 1900 linear feet from the proposed development. The gas line will be installed by National Grid and such work will be performed concurrently with the construction of the access roadway.

#### SECTION 4.0 - DETAILED DESIGN - ACCESS ROADWAY

In addition to developing a schematic design of the recreation complex, Gale was tasked with designing a roadway to provide access from Main Street to the development. The Town does not own property with frontage on Main Street. However, an adjacent undeveloped parcel owned by Bostik does have frontage on Main Street. Because the Town does not own the property that has frontage on Main Street, an Access Agreement is being drafted to provide the Town with permission to construct and use a roadway on the property owned by Bostik. The following is a discussion of the proposed roadway design. Refer to Enclosure 7 for associated plans.

#### 4.1 Layout

Because of the extensive amount of wetlands and associated buffer zones existing within the parcel, options for the layout of the roadway are limited. The road is proposed as a twenty-six foot (26') wide bituminous concrete roadway, as required by the Rules and Regulations of the Planning Board Governing the Subdivision of Land in Lynnfield, Massachusetts (Subdivision Regulations). The cross section of the roadway includes a twelve inch (12") compacted, processed gravel subbase, a two and one half inch (2 1/2") bituminous concrete pavement binder course, and a one and one half inch (1 1/2") bituminous concrete pavement top course. The road is proposed to include a Cape Cod berm along both edges of pavement, as well as a 5-foot (5') wide bituminous concrete sidewalk on its eastern side.

The roadway starts at a proposed curb cut opening in Main Street and enters the Bostik parcel known as Map 9, Lot 192, in a primarily straight and northerly direction, with a 100' radius beginning approximately 60' from the entrance on Main Street. The road continues onto the Bostik parcel, recorded as Map 5, Lot 1584, where it bends at a 136 degree radius, in a westerly direction, until it reaches the LWD parcel. Due to the extensive limits of the wetland and associated buffer zones, a wetland crossing is required where the Bostik parcel meets the LWD parcel. The road continues along the eastern property line of the LWD parcel for approximately 280', until it turns west with a 100' radius. A second wetland crossing is proposed, where the road meets the site of the active recreation development. The total length of the roadway is approximately 1,825 feet (1,825') and contains a sidewalk, two (2) wetland crossings, two (2) retaining walls and site lighting. Disturbance of these areas will require wetland replication, typically at an area ratio of 2:1 (Replication The retaining walls and utilities will be discussed in later Area: Disturbed Area). sections of this report.

#### 4.2 Grading and Drainage

The existing topography through the Bostik parcels, where the roadway is proposed to begin, is sloped at an approximate 9% slope, with the high point at Main Street and the grades decreasing in the northerly direction. This condition occurs through both Bostik properties within which the road is proposed. Through the LWD combined parcel, the slopes flatten out to between 2% and 6% and couple of small hills and valleys exist within the parcel.

The grading of the roadway is proposed to begin with the leveling of an area of approximately seventy feet (70') in length and sloped at 1.5%. The Subdivision Regulations require that the leveling of an area of at least twenty-five feet (25') in length be provided. The grade of the proposed roadway continues at a 7.5% downward slope, through the Bostik properties, complying with the requirement for a maximum slope of 9% based on the Subdivision Regulations. The slope decreases to 6% and then to 1.35% through the LWD property, where it meets the active

recreation complex. Although a 9% slope is allowed within the Town Subdivision Regulations, Gale does not recommend slopes greater than 7.5%. A slope of 6% would be ideal to achieve appropriate safety and queuing conditions.

Due to the two (2) required wetland crossings, retaining walls are proposed along the edges of roadway to limit the impact on the wetland. The larger of the two (2) retaining walls, at approximately 400' in length and eighteen feet (18') at its maximum height, is located between Station 5+00 and 9+00, along the eastern side of the road. The smaller retaining wall, along the west side of the roadway, is approximately 150' in length and fifteen feet (15') at its maximum height.

Drainage of the roadway is proposed to be achieved through a pipe network consisting of precast concrete catch basins, manholes and reinforced concrete pipe. The drainage systems will outfall to a proposed detention basin, sized to attenuate peak flows during the 2-, 10-, and 100-year storms.

The drainage system proposed within the roadway is primarily through catch basins and conveyance structures. Two (2) detention basins are proposed for stormwater collection, water quality and peak rate attenuation, prior to releasing to the existing wetlands. The first detention basin (DB-1) will be located approximately at roadway station 13+00 and will service the first 1300' of the roadway. DB-1 has been preliminarily sized for the 100-year storm event, at approximately 17,000 cubic feet in volume. An overflow spillway will be provided toward the adjacent wetlands. The other detention basin (DB-2) will be located where the roadway meets the recreation complex at station 18+00, and will service the final 550' of the roadway. DB-2 will be approximately 12,000 cubic feet in volume, preliminarily sized to accommodate the 100-year storm.

#### 4.3 Earthwork

Construction of the access roadway will require a significant amount of earthwork to achieve the leveling area and slopes described in the previous section and to be consistent with the Subdivision Regulations. Based on the earthwork volume calculations between the existing grade and the proposed roadway grade, an estimated 28,700 cubic yards of fill is required to achieve the proposed grades. If the Town were relying on the import of offsite materials, this would equate to a cost of nearly \$350,000. However, the active and passive recreation sites will also require a significant amount of earthwork to achieve proposed grades for the parking lots and athletic fields, primarily consisting of excavation and earthwork cuts. There are several drumlins or hills located throughout the property where the recreation complex is proposed. Based on test pits performed throughout the site, the materials within six to eight feet (6'-8') of existing grade appear to be sandy, well-draining material and will likely be suitable for use as common fill. proposed recreation complex site was graded schematically, with the intent to achieve a balanced site. The final cut/fill analysis of the site, including the recreation areas and the roadway areas, results in a net cut of 5,600 cubic yards,

resulting in excess material. Therefore, it is recommended that, prior to the start of construction of the access roadway, the recreation areas be brought to the proposed subgrade to allow the Town to use the excavated material from the recreation sites for fill beneath the proposed roadway. This will require significant tree clearing, erosion control and stabilization measures. During construction, the excavated materials will be tested and approved as suitable fill for the subgrade of the roadway prior to installation.

#### 4.4 Utilities

As described in previous sections, no utility services are currently provided to the undeveloped LWD parcel. As discussed, electric service, natural gas and storm drainage utilities will be installed during construction of the roadway. In addition, the utility requirements for athletic lighting, irrigation and potable water, and the amenities building, roadway and sidewalk will be required to contain site lighting. Single phase electric service is proposed to be provided for the site lighting requirements. Refer to Section 3.5 for additional information on utility provisions for the proposed development.

#### 4.5 Traffic Impact Assessment

A Traffic Impact Assessment was conducted by Vanasse & Associates, Inc. (VAI) for Main Street to determine potential traffic impacts caused by the Main Street Recreation Complex access drive and parking areas. Refer to Enclosure 8 for the full Traffic Assessment Report. It was determined that the proposed facility will create an additional 934 vehicle trips on the average weekday and 294 vehicle trips on weekends and weekday afternoons. It is estimated that these additional trips will create a minimal delay of fifteen (15) seconds or less, with vehicle queuing of zero to one (0-1) cars at the intersection of the proposed driveway and Main Street during peak operation hours. Using a traffic analysis software, and the 2018 build conditions, it was determined that the proposed facility would operate under a "B" Grade Level of Service for normal field utilization. It is recommended that a plan be created with the Lynnfield Police Department for any tournament or special event uses where traffic conditions may require a police traffic detail.

A parking assessment for the facility was conducted during the study to determine the minimum required parking demand. Parking demand calculations were conducted in accordance with the Institution of Traffic Engineers (ITE) land use code for the average 85th percentile parking demands of soccer fields at similar facilities. Based on the 85th percentile, an average of 60.5 parking spaces is required per field, with a total of five (5) fields assumed (including tennis and basketball). The calculated 85th percentile peak parking demand, assuming all five fields are in use simultaneously for the weekday/afternoon period, is 302 spots and 326 spots for the Saturday mid-day peak period. The proposed facility includes 339 total parking spots, which exceeds the maximum 85th percentile requirement of 326 spots.

The proposed access road for the facility was analyzed for sight line safety at the intersection of Main Street and for the safety and size of the proposed roadway. An average vehicle speed study was conducted on Main Street for both northbound and southbound traffic. The current posted speed limit for northbound and southbound traffic is 25 miles per hour (mph) and 35 mph, respectively. The 85th percentile actual vehicular speed for northbound and southbound traffic is 39 mph and 42 mph, respectively. The 85th percentile travel speed is used for sight line minimums for safety purposes. Based on a speed of 45 mph, the driveway access road would require a 360 foot (360') sight line for both northbound and southbound traffic. The sight lines at the proposed curb cut were measured to be 360 feet (360') for northbound traffic and 650 feet (650) for southbound traffic. The proposed sightlines both meet or exceed the minimum required for vehicular travel at 45 mph. For safe travel to and from the facility at Main Street, it is recommended that the access drive be a minimum of twenty-four feet (24') in width, with a leveling area not to exceed 2% in grade, with a minimum distance of 100 feet. These recommendations have been met.

#### SECTION 5.0 - PERMITTING IMPLICATIONS

The Main Street Recreation Complex will require permitting through the Lynnfield Board of Appeals and Conservation Commission. A summary of the authorities having jurisdiction is tabulated below. A discussion on the permitting follows.

Lynnfield Zoning Board	of Appeals	
Site Plan Approval		Town of Lynnfield Zoning Bylaws require that Site Plan Approval be submitted and approved by the Zoning Board of Appeals for uses that do not conform to permitted uses. While public recreational use is permitted within Single Residence District D, it is anticipated that Site Plan Approval will be required due to the size of the project, parking requirements and proposed change in use.
<b>Conservation Commissi</b>	on	
No-Disturb Zone	25'	There shall be a No-Disturb Zone with a minimum depth of twenty-five (25) feet measured horizontally from the border of the Resource Area. Vegetation in the No-Disturb Zone shall not be cut or trimmed in any manner. Prohibited activities within the No-Disturb Zone include grading, landscaping, planting, harvesting, mowing, clearing, cutting, trimming, filling, depositing, composting, excavating, construction, fencing, and installation of roads, driveways and walkways.

No-Build Zone	50'	There shall be a No-Build Zone with a minimum depth of fifty (50) feet measured horizontally from the border of the Resource Area. Prohibited activities within the No-Build Zone include, but are not limited to, construction of any structure, installation of any impervious surface and any work requiring a building permit.
Massachusetts Departm	ent of Env	ironmental Protection (MA DEP)
Buffer Zone	100'	Any activity other than minor activities identified in 310 CMR 10.02(2)(b)1., proposed or undertaken within 100 feet of an area specified in 310 CMR 10.02(1)(a), which will alter an Area Subject to Protection, is subject to regulation and requires the filing of a Notice of Intent. Minor Activities: unpaved pedestrian walkways for private use, fencing, pruning, planting of native trees, etc.
Ipswich River Watershee	d Associat	ion (IRWA)
Ipswich River Watershed Association (IRWA)		IRWA's policy and advocacy efforts focus on ensuring that state water policies, water withdrawal permits and regulatory decisions are protective of ecosystem health and include prudent water conservation measures. IRWA also comments on proposed projects that have a significant potential to impact the Ipswich River.

#### 5.1 Zoning Board of Appeals / Planning Board

Submission for an application for Site Plan Approval will likely be required, although the regulations are somewhat vague regarding what triggers submission of a Site Plan Approval Application. However, typical projects of this size that impact traffic, use, stormwater, parking, etc. require filing with the Planning Board and/or Zoning Board of Appeals. We recommend that an informal pre-application meeting be scheduled with the Planning Board and/or Zoning Board of Appeals to determine what the requirements will be in regards to their jurisdiction over the project.

#### 5.2 Conservation Commission / MA DEP

Due to the extensive amount of bordering vegetated wetlands and the associated 25', 50' and 100' buffers, filing of a Notice of Intent (NOI) with the Conservation Commission will certainly be required. There are also two (2) resource areas, the Ipswich River and the Mills Brook, which have a 200' Riverfront Protection Buffer associated with them. In addition to the work that is proposed within the 100' buffer that will require filing of an NOI, the design proposes to create wetland crossings, which requires filling a portion of the wetlands. This activity certainly requires filing of an NOI with the Conservation Commission and the Massachusetts Department of Environmental Protection (MADEP).

Prior to filing a NOI, an Abbreviated Notice of Resource Area Delineation (ANRAD) is required to be submitted to the Conservation Commission to provide the Commission with data regarding the wetland delineation process that was followed in the delineation of the wetlands. The Commission reviews the information, hires a third-party reviewer to review the delineation and holds hearings to determine if the delineation will be accepted for a period of three (3) years or if changes need to be made to the wetland delineation prior to its acceptance and approval.

Gale submitted an ANRAD and presented the information at the initial public hearing, in front of the Conservation Commission, on December 11, 2012. The Fields Committee has since decided to postpone the ANRAD process until a decision is made whether or not development of the parcel will be pursued. This was done in an effort to save money required for the fee for the third-party reviewer, if in the event this project does not go forward. Please refer to Enclosure 5 for the ANRAD application.

#### 5.3 Ipswich River Watershed Association (IRWA)

The Ipswich River is located in the northeast corner of the parcel. Associated with the river is a 200' Riverfront Protection Buffer. Although the proposed design is not within this buffer zone, which is under the jurisdiction of the MADEP, we recommend that a meeting be held with the IRWA to determine if there is any jurisdiction over the project and its components. It is likely that, although the IRWA advocates for the protected rivershed, the authorities having jurisdiction are the MADEP and the Lynnfield Conservation Commission. An informal meeting with the IRWA will be scheduled to provide an overview of the project and allow the IRWA to provide any feedback or comments that may affect the development.

#### SECTION 6 - ESTIMATED COSTS OF CONSTRUCTION

A schematic level cost estimate was created for both Option 1 and Option 2 of the Main Street Recreation Parcel. Both cost estimates have been broken down into sections for the active recreation areas, passive recreation areas and construction of the access roadway. Because the cost estimates are schematic, they are subject to change with the evaluation design, development and construction.

Option 1 is estimated to cost \$6,189,000, including the active recreation area, the passive recreation area and the access roadway. The active recreation area for Option 1, which includes a synthetic turf field with lights, two (2) natural turf fields, one (1) basketball court, an amenities building, one (1) playground with two (2) play areas, three (3) parking lots, access drives and landscaping across the entire area, would cost approximately \$4,050,000. The passive recreation area, which includes a trail network, a dog park, an

open space and an access drive, would cost approximately \$764,000. The main access drive to the facility would cost approximately \$1,375,000. The access drive is such a significant cost of the project due to the length of the road (1850'), the amount of earthwork required, wetland crossings, retaining walls and utility runs. While earthwork is a significant portion of the cost, a savings is provided through use of fill materials from the recreation site.

Option 2 is estimated at approximately \$6,675,220, including the active recreation area, the passive recreation area and the access roadway. The active recreation area for Option 2, which includes two (2) synthetic turf fields with lights, one (1) natural turf field, one (1) basketball court, an amenities building, one (1) playground with two (2) play areas, three (3) parking lots, access drives and landscaping across the entire area, would cost approximately \$4,537,000. Identical to Option 1, the passive recreation area, which includes a trail network, a dog park, parking and an access drive, would cost approximately \$764,000 and the main access roadway would cost approximately \$1,375,000.

Refer to Enclosure 9 for detailed schematic cost estimates of the proposed Recreation Complex.

#### SECTION 7 - CONCLUSIONS AND RECOMMENDATIONS

The LWD parcels located off Main Street in Lynnfield, MA consist of approximately 98.5 acres of forested land, with well-draining sandy soils, varying topography, moderate depths to groundwater and an extensive amount of wetland resource areas. With the exception of the substantial amount of wetland areas and the lack of right-of-way frontage, the site is essentially ideal for development of a recreation complex. However, the extensive resource areas and the earthwork, wetland crossings and area associated with the proposed access roadway from Main Street, may make this development cost prohibitive for the Town.

The area of development proposed for the recreation complex affords the Town an opportunity to develop several new MPR fields, hard court space, a seasonal skating area, an extensive trail network, gardening plots, open space and a dog park to accommodate the recreational needs of the Town. Based on the results of the Master Plan, this would be the ideal development to meet the Town's needs relating to recreational demand.

Gale recommends that if the Town is not able to pursue development of the Main Street Recreation Complex, whether due to access, wetland resource areas and/or associated costs, an alternative location should be researched for development of a recreation complex to achieve the components of the planning program proposed at the Main Street Complex. The Master Plan provides details of the recreational demands, needs assessment survey results and proposed developments. While the Main Street Recreational Complex may not be feasible for development at the LWD Main Street Parcel, a feasibility study should be completed at an alternative location.

# Enclosure 1 Existing Conditions Plan



**EAGLEBROOK** 

91 MAPLE STREET, SUITE 304 DANVERS, MASS, 01923 TEL: (978) 777-0494

LYNNFIELD, MASSACHUSETTS
PREPARED FOR
GALE ASSOCIATES, INC.
163 LIBBEY PARKWAY
WEYMOUTH, MA.

STAMP: 10/25/12

OCTOBER 10, 2012

REVISIONS:

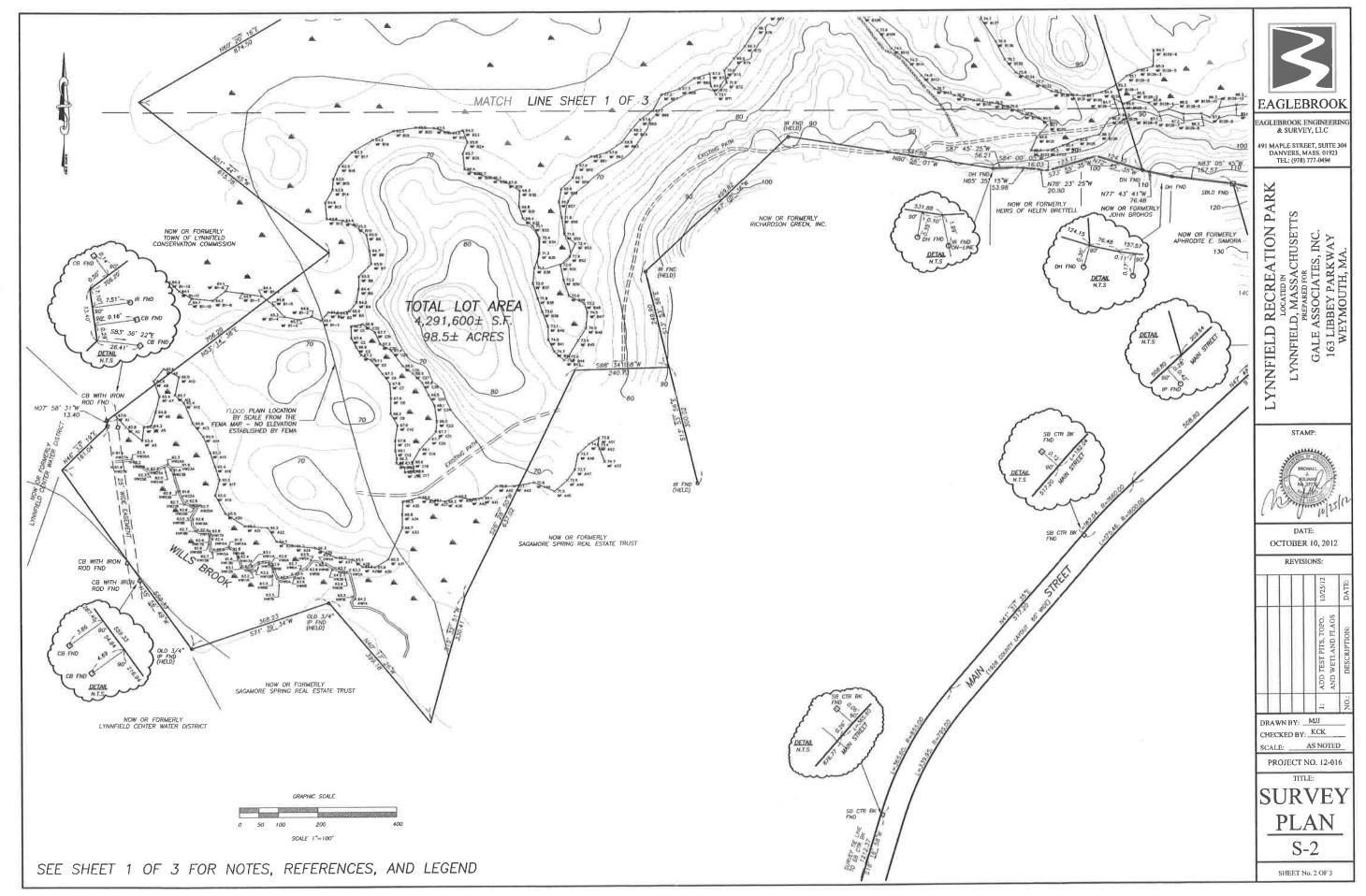


DRAWN BY: MJJ CHECKED BY: KCK

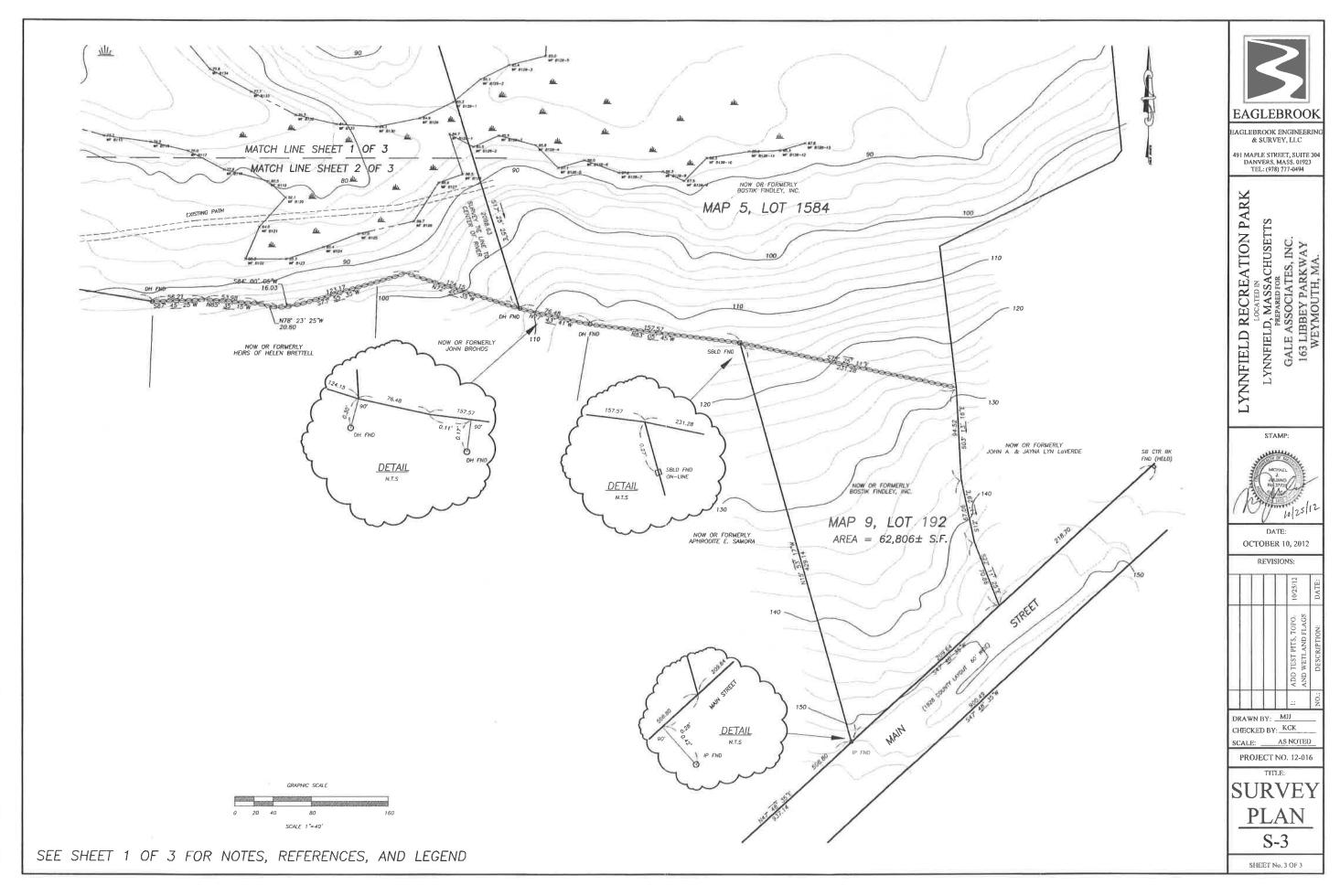
PROJECT NO. 12-016

**SURVEY PLAN** 

SHEET No. 1 OF 3

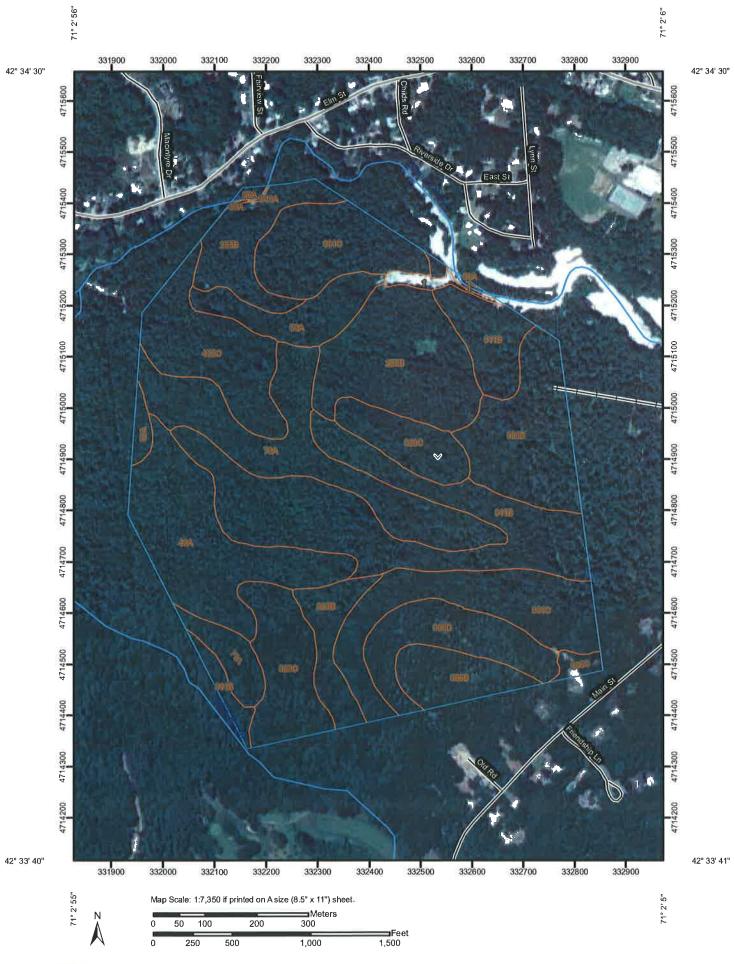


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glebrook Projects/12-016 Gule Assoc Lynnfield/DWG/PROPERTY LINE PLAN 40 SCALE.dwg. 10:25:2012 2:3

# Enclosure 2 NRCS Web Soil Survey



# MAP LEGEND

#### Streams and Canals Short Steep Slope Very Stony Spot Special Line Features Wet Spot Other Other Gully Cities Political Features Water Features **Fransportation** 8 Area of Interest (AOI) Closed Depression Soil Map Units Special Point Features Gravelly Spot **Вогтоw Pit Gravel Pit** Area of Interest (AOI) Clay Spot Blowout Landfill X Э Soils

Ħ





Miscellaneous Water

Perennial Water

Rock Outcrop



# JS Routes

Marsh or swamp

Lava Flow

Mine or Quarry



Severely Eroded Spot

Slide or Slip

Sinkhole

Sodic Spot

Stony Spot

Spoil Area

Sandy Spot Saline Spot

Warning: Soil Map may not be valid at this scale.

The soil surveys that comprise your AOI were mapped at scales

ranging from 1:15,840 to 1:25,000.

Map Scale: 1:7,350 if printed on A size (8.5" × 11") sheet.

MAP INFORMATION

misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting Enlargement of maps beyond the scale of mapping can cause soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for accurate map measurements.

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Source of Map: Natural Resources Conservation Service Coordinate System: UTM Zone 19N NAD83 This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Essex County, Massachusetts, Southern Part Version 9, Feb 26, 2010 Survey Area Data: Soil Survey Area:

Middlesex County, Massachusetts Survey Area Data: Version 12, Feb 26, 2010 Soil Survey Area:

interpretations that do not completely agree across soil survey area a different land use in mind, at different times, or at different levels Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with of detail. This may result in map unit symbols, soil properties, and boundaries.

Date(s) aerial images were photographed: 7/7/2003

imagery displayed on these maps. As a result, some minor shifting The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background of map unit boundaries may be evident.

# **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres In AOI	Percent of AOI
1	Water	1.0	0.5%
32A	Wareham loamy sand, 0 to 3 percent slopes	1.0	0.5%
43A	Scarboro mucky loamy fine sand, 0 to 1 percent slopes	17.4	9.4%
53A	Freetown muck, ponded, 0 to 1 percent slopes	6.2	3.3%
71A	Ridgebury fine sandy loam, 0 to 3 percent slopes, extremely stony	3.2	1.7%
73A	Whitman loam, 0 to 3 percent slopes, extremely stony	17.2	9.3%
255B	Windsor loamy sand, 3 to 8 percent slopes	23.1	12.5%
301C	Montauk fine sandy loam, 8 to 15 percent slopes, very stony	24.1	13.0%
306B	Paxton fine sandy loam, 3 to 8 percent slopes, very stony	5.5	2.9%
306C	Paxton fine sandy loam, 8 to 15 percent slopes, very stony	1.0	0.5%
306D	Paxton fine sandy loam, 15 to 25 percent slopes, very stony	9.1	4.9%
311B	Woodbridge fine sandy loam, 3 to 8 percent slopes, very stony	32.7	17.6%
323B	Poquonock loamy sand, 3 to 8 percent slopes, very stony	17.7	9.5%
323C	Poquonock loamy sand, 8 to 15 percent slopes, very stony	14.3	7.7%
422C	Canton fine sandy loam, 8 to 15 percent slopes, extremely stony	11.4	6.1%
616A	Fluvaquents, frequently flooded, 0 to 3 percent slopes	0.1	0.0%
Subtotals for Soil Surv	vey Area	184.9	99.7%
Totals for Area of Inte	rest	185.5	100.0%

Middlesex County, Massachusetts (MA017)					
Map Unit Symbol	Map Unit Name	Acres In AOI	Percent of AOI		
1	Water	0.3	0.1%		
36A	Saco mucky silt loam, 0 to 1 percent slopes	0.3	0.2%		
Subtotals for Soll Surv	ey Area	0.6	0.3%		
Totals for Area of Inter	est	185.5	100.0%		

# Enclosure 3 Geotechnical Test Pit Logs



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review (Continued)

Deep Observation Hole Number:

1 (Refer to Existing Conditions)

4,000	Soil Horizon/	Soil Horizon/ Soil Matrix: Color-	Redox	Redoximorphic Features (mottles)	atures	Soil Texture	Coarse F % by V	Coarse Fragments % by Volume		Soil	Other
Deptn (ini.)	Layer	Moist (Munsell)	Depth	Color	Percent	(USDA)	Gravel	Cobbles & Stones	Structure	(Moist)	
3-0	0	10yr3/1	ń	•	2003	loam	ı	ā	blocky	friable	
9-0	Bw1	10yr5/8	r	•	(t)	sandy loam	9.40	à.	gr	fr/loose	
6-18	Bw2	10yr5/8	ř	i)	ij.	med sand	•	20%		loose	
18-30	Bw3(BC)	10yr4/6	î	ř	i	fine sand	ŧ	10%	á <b>n</b>	esool	
30-96	C1	10yr6/4	ř	ľ	Û	fine sand		25%	36	esool	

Additional Notes:

No standing water, weeping, or

large rocks 18"-30" between B.



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

## C. On-Site Review (Continued)

2 (Refer to Existing Conditions Plan)

Deep Observation Hole Number:

ii oo	Soll Soldstone	Consistence Other (Moist)	onsistence Other (Moist) friable	onsistence Other (Moist) friable friable	onsistence Other (Moist) friable friable frim	friable frrm	friable firm firm	friable frrm
Soil	Structure	4)	gr	gr fr	gr fr blocky/gr fr dense in place	gr fr blocky/gr fr dense in place dense in place	gr fr blocky/gr fr dense in place dense in place	gr fr blocky/gr fr dense in place dense in place
% by Volume	Cobbles & Stones		10%	10%	10% 20% 25%	10% 20% 25% 25%	10% 20% 25% 25%	10% 20% 25% 25%
	Gravel		700	1( <b>0</b> ).	00 t P	10: 6: 6: 6:	10 C C C	
Soil Texture	(USDA)		loamy	loamy loamy sand	loamy loamy sand fine-med sand	loamy sand fine sand fine sand	loamy loamy sand fine-med sand fine sand	loamy sand fine-med sand fine sand
	Percent		•	ı				
(mottles)	Color	•			E .			
7	Depth	10		î	i i	r r r	1 1	r r r
Soil Horizon/Soil Matrix: Color-	Moist (Munsell)	10yr3/1		10yr5/6	10yr5/6 10yr6/4	10yr5/6 10yr6/4 10yr6/3	10yr5/6 10yr6/4 10yr6/3	10yr5/6 10yr6/4 10yr6/3
Soil Horizon/	Layer	0		Bw1	Bw2	Bw2 C1	Bw2 C1	Bw1 Bw2 C1
;	Depth (in.)	3-0		2-0	0-7	7-25	7-25	7-25

Additional Notes:

Very bonay, dense - backhoe robial @ 71 No vidox, standing water, or weeping Bw. - C. : bose in hand,



## Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review (Continued)

Deep Observation Hole Number:

3 (Refer to Existing Conditions Plan)

r						-	
Ş							
Soil	(Moist)	fr/loose	loose	firm	loose		
	Structure	gr	blocky/gr	boney/ dense	dense		
Coarse Fragments % by Volume	Cobbles & Stones		10%	20-30%	9		
	Gravel	940	96:	-	Ľ		
Soil Texture	(USDA)	loam	loamy sand	fine sand	fine sand		
	Percent	(OB)	<b>I</b> S	ľ	10		
Redoximorphic Features (mottles)	Color	-	1	U	Ŀ		
	Depth	40	ı.	i)	140		
Soil Horizon/Soil Matrix: Color-	Moist (Munsell)	10yr3/1	10yr6/6	10yr6/4	10yr7/4		
Soil Horizon	Layer	0	Bw1	Bw2	PS		
1	Deptin (in.)	8-0	8-20	20-30	30-		

Additional Notes:

Very cobblerg, large stones, compact glacial fill

No groundwater, redox, or weeping

2 boulders > 1 cy at bottom

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal • Page 3 of 8



## Commonwealth of Massachusetts

City/Town of

# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

## C. On-Site Review (Continued)

Deep Observation Hole Number:

4 (Refer to Existing Conditions Plan)

Soil	(Moist)	fr	loose	loose	loose		
Soil	Structure	gr	single grain	firm	firm		
Coarse Fragments % by Volume	Cobbles & Stones	I.		%5-0	2-10%		
	Gravel	ř.	t	ř	ì		
Soil Texture	(USDA)	sandy loam	loams sand	fine sand	fine-med sand		
atures	Percent		Ť	9	1		
Redoximorphic Features (mottles)	Color	Ĭ	Ř	Ī	î		
	Depth	ì	Si.	7	1		
Soil Horizon/Soil Matrix: Color-	Moist (Munsell)	10yr3/2	10yr5/8	10yr7/2	10yr6/2		
Soil Horizon/	Layer	0	Bw1	Bw2	8		
:	Depth (in.)	0-10	10-28	28-75	75-102		

Additional Notes:

No ledge observed, but very dense, boney, + or way

different. Clear distriction be Box + Cd

No standing water, redox, or weeping



## Commonwealth of Massachusetts

City/Town of

# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review (Continued)

Deep Observation Hole Number:

5 (Peter to Existing Conditions Plan)

3							
Soil	re (Moist)	esool	loose	loose	loose		
Soil	Structure	single grain	single grain	single grain	firm		
Coarse Fragments % by Volume	Cobbles & Stones	36	-	<b>1</b>	10%		
Coarse F % by V	Gravel	<b>(#</b> S	41:	Ē	3₹		
Soil Texture	(USDA)	loam/o	loamy sand	fine beach sand	fine-med beach		
atures	Percent	æ	1	17	ŗ		
Redoximorphic Features (mottles)	Color	90	Ng)	E)	<b>*</b>		
	Depth	(8)	3	-			
Ponth (in ) Soil Horizon/Soil Matrix: Color-	Moist (Munsell)	10yr3/2	10yr6/8	10yr7/2	10yr5/2		
Soil Horizon/	Layer	Of	Bw1	Bw2	Cd		
Donth (in )	Ceptal (iii.)	9-0	6-20	20-55	55-108		

Additional Notes:

Loose until Cd, which is very device

No redox, stanting water, or weeping

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal • Page 3 of 8



## Commonwealth of Massachusetts

## City/Town of Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review (Continued)

Deep Observation Hole Number:

6 (Refer to Existing Conditions Plan)

Other							
Soil	(Moist)	loose	loose	loose	1		
Soil	Structure	gr	firm	single grain	dense		
Coarse Fragments % by Volume	Cobbles & Stones	:( <b>+</b> )	2%	10%	10%		
Coarse F % by V	Gravel	ı	3	ж	91		
Soil Texture	(USDA)	crunchy	fine sand	fine sand	fine sand		
atures	Percent		4	a	X <b>1</b>		
Redoximorphic Features (mottles)	Color	•	1	.it	1		
	Depth	ı	Ų.	,	31		
Soil Matrix: Color-	Deptn (In.) Layer Moist (Munsell)	10yr2/1	10yr5/4	10yr7/2	10yr8/2		
Soil Horizon/	Layer	0	Bw1	BC	3		
	Deptin (in.)	0-24	24-40	40-46	46-96		

Additional Notes:

Very thick O layer - inconsistent

Many large boulders

an industry observed exavator, No Dance



# Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

### C. On-Site Review (Continued)

7 (Peter DEXISTING Conditions Plan)

Deep Observation Hole Number:

_						 	
Other							
Soil	(Moist)	Ψ	esool	firm	firm		
Soil	Structure	gr	single grain	single grain	single grain		
Coarse Fragments % by Volume	Cobbles & Stones	¥.	2%	20%	boulders at refusal		
Coarse Fi % by V	Gravel		*	•			
Soil Texture	(USDA)	loam	loamy sand	fine-med sand	fine-med sand		
atures	Percent	<b>(</b>		91	36		
Redoximorphic Features (mottles)	Color		,	(0)	i		
	Depth	1	\$ <b>(</b> ()	T.	•		
Soil Matrix: Color-	Depth (in.) Layer Moist (Munsell)	10yr4/2	10yr6/6	10yr6/3	10yr6/2		
Soil Horizon/	Layer	0	Bw1	8	3		
	Depth (in.)	0-12	12-30	30-54	54-84		

Additional Notes:

757

redox, standing water, or weeping ž

6				

### ${\bf Enclosure} \ 4$ ${\bf Environmental} \ {\bf Resource} \ {\bf Assessment} \ {\bf Report}$



November 20, 2012

### **Email**

John M. Perry, P.E. Gale Associates, Inc. 163 Libbey Parkway Weymouth, MA 02189

Re: Wetland Resource Area Analysis

Lynnfield Recreation Park
Land off Main Street
Lynnfield, Massachusetts

Dear Mr. Perry:

LEC Environmental Consultants, Inc., (LEC) is pleased to submit this Wetland Resource Area Analysis Report documenting methods and findings associated with a wetland delineation conducted on several lots located on Main Street in Lynnfield, Massachusetts. The purpose of the wetland delineation was to identify the boundaries of Wetland Resource Areas protected under the *Massachusetts Wetlands Protection Act* (M.G.L. c. 131, s. 40), its implementing *Regulations* (310 CMR 10.00), and the *Lynnfield Environmental Bylaw* (Chapter 9) and associated *Rules and Regulations*.

The subject property boundaries, site topography, and wetland flagging locations are depicted on the *Lynnfield Recreation Park Plan*, prepared by Eaglebrook Engineering and Surveying, LLC, dated October 25, 2012.

### 1. General Site Description

The site consists of several undeveloped parcels (see Note 4 on the *Plans* for Assessor's reference) totaling approximately 98.5 acres located west off Main Street in the northern portion of Lynnfield, Massachusetts. The parcels are owned by the Town of Lynnfield Center Water District, with the exception of Map 9, Lot 192 and Map 5, Lot 1584 which are privately owned. Map 9, Lot 192 contains frontage on Main Street and does not contain any Wetland Resource Areas.

The site includes extensive forested uplands and forested wetlands with a network of cart paths and trails traversing the site. ATV use of the cart paths and trails is evident. An abandoned railroad easement extends across the northern portion of the property. The northern limits of the site extend to the centerline of the Ipswich River while Will's Brook flows across the southern portion of the site. Each of these rivers is identified as a perennial stream.

Property to the east consists of undeveloped uplands and scattered single-family dwellings along Main Street, while an industrial building exists to the northeast. Property to the west is primarily town-owned undeveloped forested wetlands and uplands while property to the south/southeast is occupied by a golf course.

PEYMOUTH

WAKEFIELD

508-753-3077

WORCESTER

www.lecenvironmental.com

[LEC File #: GAI\12-224.01]



The forested upland is defined by a mixed canopy of red oak (*Quercus rubra*), white oak (*Quercus alba*), eastern white pine (*Pinus strobus*), and red maple (*Acer rubrum*), with scattered individuals of sassafras (*Sassafras albidum*) and swamp tupelo (*Nyssa sylvatica*). The upland understory is comprised of witch hazel (*Hammamelis virginiana*), sweet pepperbush (*Clethra alnifolia*), highbush blueberry (*Vaccinium corymbosum*), dangleberry (*Gaylussacia frondosa*), black huckleberry (*Gaylussacia baccata*), and arrowwood (*Viburnum dentatum*), with entanglements of greenbrier (*Smilax rotundifolia*). Groundcover consists of tree clubmoss (*Lycopotium obscurum*), teaberry (*Gaultheria procumbens*), and hay-scented fern (*Dennstaedtia punctilobula*).

According to the Essex County Soil Survey, the site includes areas mapped as Whitman loam, extremely stony; Scarboro mucky loamy fine sand; Windsor loamy sand; Paxton fine sandy loam, very stony; Woodbridge fine sandy loam, very stony; and Poquonock loamy sand, very stony. Overall the site is defined by glacial till with stony soils and scattered boulders at the surface.

### 1.1 Floodplain Designation

According to FEMA Flood Insurance Rate Map for the Town of Lynnfield dated July 3, 2012 (Community Panel 25009C0383F), the site contains extensive areas mapped as Zone A, Areas subject to inundation by the 1% annual chance flood. The extent of Zone A is roughly coincident with the extent of BVW on the property. The upland portions of the property are mapped as Zone X, Areas determined to be outside the 100-year flood plain.

### 1.2 Massachusetts Natural Heritage & Endangered Species Program Designation

According to the 13<sup>th</sup> edition of the Massachusetts Natural Heritage Atlas (valid from October 1, 2008) published by the Natural Heritage & Endangered Species Program (NHESP), the property is not located within an *Estimated or Priority Habitat for Rare Species*. According to the MassGIS datalayers, there are no certified or potential vernal pools on the site.

### 2. Wetland Delineation Methodology

The extent of the Bordering Vegetated Wetlands (BVW) was determined through observations of the existing plant communities, using the "fifty percent criteria" to determine dominance of wetland/upland vegetation, the interpretation of soil characteristics, and other indicators of wetland hydrology in accordance with the criteria enumerated within 310 CMR 10.55 (2), the Handbook prepared by the Massachusetts Department of Environmental Protection, entitled *Delineating Bordering Vegetated Wetlands Under the Massachusetts Wetlands Protection Act* (March 1995), as well as the *Field Indicators for Identifying Hydric Soils in New England* (April 2004). The boundaries are demarcated in the field with blaze orange surveyor's flagging tape embossed with the words "LEC Resource Area Boundary" in bold, black print and numbered A1-A53, B1-B193, C1-C32, D1-D63, and E1-E39.

In addition, LEC demarcated the Mean Annual High Water (MAHW) to the Ipswich River and Will's Brook in accordance with 310 CMR 10.58 (2) (a) (2). MAHW was demarcated with blue flagging tape numbered MAHW 1-4 along the Ipswich River and MAHW 1A-27A/1B-27B along Will's Brook.

Page 2 of 4



### 3. Wetland Resource Area Descriptions

Wetland Resource Areas located on or adjacent to the property include BVW, Riverfront Area, and Bank. The site may also contain Bordering Land Subject to Flooding (BLSF) as described below. A brief description of each Wetland Resource Area follows.

### 3.1 BVW

BVW is defined at 310 CMR 10.55 (2) (a) as freshwater wetlands that border on creeks, rivers, streams, ponds and lakes. The types of freshwater wetlands are wet meadows, marshes, swamps and bogs. Bordering Vegetated Wetlands are areas where the soils are saturated and/or inundated such that they support a predominance of wetland indicator plants.

The property contains extensive forested BVW areas associated with a series of intermittent streams as well as the perennial streams. The BVW canopy typically consists of red maple (*Acer rubrum*) and eastern white pine (*Pinus strobus*), with scattered individuals of red oak (*Quercus rubra*), white oak (*Quercus alba*), and swamp tupelo (*Nyssa sylvatica*). The understory is variably dense, with some very dense areas dominated by sweet pepperbush, while other areas are more open allowing conspicuous patches of cinnamon fern (*Osmunda cinnamomeo*), goldthread (*Coptis trifolia*), and sphagnum moss (*Sphagnum spp.*) to thrive. Other species observed in the BVW include highbush blueberry (*Vaccinium corymbosum*), arrowwood (*Viburnum dentatum*), swamp azalea (*Rhododendron viscosum*), witch hazel (*Hamamelis virginiana*), royal fern (*Osmunda regalis*), and sensitive fern (*Onoclea sensibilis*).

The C-series wetland flagging is a smaller, seemingly isolated system separated from the larger BVW areas. The wetland does contain evidence of flow that may be considered an intermittent stream; however, the overland does not appear to have created an incised channel and thus categorizing the area as BVW (as we have) bordering on an intermittent stream may be disputed. In any case, the Lynnfield *Bylaw* protects isolated wetlands so the C-series wetland is a protected Wetland Resource Area under that statute regardless of whether or not it borders on a stream.

### 3.2 Riverfront Area

According to 310 CMR 10.58(2)(a)(3), Riverfront Area is defined as the area of land between a river's mean annual high-water line measured horizontally outward from the river and a parallel line located 200 feet away.

The Mean Annual High-Water (MAHW) Line to perennial streams is delineated in the field to establish the limits of Riverfront Area. MAHW was delineated in accordance with definition in 310 CMR 10.58 (2)(a)2. - the line that is apparent from visible markings or changes in the character of soils or vegetation due to the prolonged presence of water and that distinguishes between predominantly aquatic and predominantly terrestrial land. Field indicators of bankfull conditions shall be used to determine the mean annual highwater line. Bankfull indicators include but are not limited to: changes in slope, changes in vegetation, stain lines, top of pointbars, changes in bank materials, or bank undercuts.

Riverfront Area is associated with a small segment of the Ipswich River on the northern portion of the site and with Will's Brook on the southern portion of the property. In each case, the 200-foot Riverfront Area comprises forested uplands and forested wetlands.

Page 3 of 4



### 3.3 Bank

Bank is defined at 310 CMR 10.54(2)(a) as the portion of land surface which normally abuts and confines a water body. The upper boundary of a bank is the first observable break in the slope or the mean annual flood level, whichever is lower. The lower boundary of a bank is the mean annual low flow level.

Bank is associated with the perennial streams described above and is coincident with the delineated MAHW line. Additional Bank is associated with numerous intermittent streams contained within the BVW.

### 3.4 Bordering Land Subject to Flooding

According to 310 CMR 10.57(2)(a)(1), BLSF is an area with low, flat topography adjacent to and inundated by flood waters rising from creeks, rivers, streams, ponds or lakes. It extends from the banks of these waterways and water bodies; where a bordering vegetated wetland occurs, it extends from said wetland.

BLSF exists in those portions of the property where the 100-year flood plain extends beyond the BVW boundary (i.e. into the upland). Further review of the actual extent of the flood plain would be necessary to determine the extent of BLSF on the property.

### 4. Regulatory Implications

The site contains Bordering Vegetated Wetlands, Bank, and Riverfront Area; Wetland Resource Areas protected under the *Massachusetts Wetlands Protection Act* (M.G.L. c. 131, s. 40), its implementing *Regulations* (310 CMR 10.00), and the *Lynnfield Environmental Bylaw* (Chapter 9) and associated *Rules and Regulations*. As described above, the site may also contain BLSF. Any proposed activity or alteration within the above-referenced Wetland Resource Areas or their Buffer Zones will require filing the necessary permit applications with the Lynnfield Conservation Commission and the Massachusetts Department of Environmental Protection (DEP).

Sincerely,

LEC Environmental Consultants, Inc.

Mark L. Manganello

Assistant Director of Ecological Services

Mark L Manganello (ss)

9			

### ${\bf Enclosure~5}$ Abbreviated Notice of Resource Area Delineation (ANRAD) Application



### Abbreviated Notice of Resource Area Delineation Town of Lynnfield, MA

November 21, 2012

### table of contents

Section 1.0	WPA Form 4A – Abbreviated Notice of Resource
	Area Delineation

Section	2.0	Property	Description
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Section 5.0 USGS Map

Section 6.0 Certified Abutters List

### enclosures

Enclosure 1 – Survey Plan Sheet S-1

Enclosure 2 - Survey Plan Sheet S-2

Enclosure 3 - Survey Plan Sheet S-3



Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return

### Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

### WPA Form 4A – Abbreviated Notice of Resource Area Delineation

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided I	by	MassDEP:
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**Document Transaction Number** 

¬:	6 [	Tovern
اب	LV/	Town

A. Genera	l Ir	nfor	ma	tio	n
-----------	------	------	----	-----	---

	N/A	LYNNFIELD	01940
	a. Street Address	b. City/Town 42d34'12.69" N	c. Zip Code 71d02'26.75" W
	Latitude and Longitude:	d. Latitude	e. Longitude
	See attached list	See attached li	st
	f. Assessors Map/Plat Number	g. Parcel /Lot Number	
•	Applicant:		
	Bill	Gustus	
	a. First Name Town of Lynnfield	b. Last Name	
	c. Organization 55 Summer Street		
	d. Mailing Address Lynnfield	MA	01940
	e. City/Town 781-334-9410 781-334-9419	<pre>f. State   william-gustus@town.l</pre>	g. Zip Code ynnfield.ma.us
	h. Phone Number i. Fax Number	j. Email Address	
<b>.</b>	Property owner (if different from applicant):	Check if more than o sheet with names and co	ne owner (attach additiona ntact information)
	a. First Name Lynnfield Center Water District	b. Last Name	
	c. Organization 842 Salem Street		
	d. Mailing Address		
	Lynnfield	MA.	01940
	B. City/Town	f. State lwd@lwdma.us	g. Zip Code
	781-598-4223 781-598-8819 h. Phone Number I. Fax Number	j. Email Address	
	h. Phone Number  I. Fax Number  Representative (if any):	j. Email Address	
	John	Perry	
	a. Contact Person First Name	b. Contact Person Last Name	
	Gale Associates, Inc.		
	c. Organization		
	163 Libbey Industrial Parkway		
	d. Mailing Address		
	Weymouth	MA	02189
	e. City/Town	f. State	g. Zip Code
	781-335-6465 781-335-6467	JMP@gainc.com	
	h. Phone Number i. Fax Number	j. Email Address	
5.	Total WPA Fee Paid (from attached ANRAD	Wetland Fee Transmittal Form	):
			\$0
	\$0 \$0	1	

b. State Fee Paid

Fees will be calculated for online users.

Note: Before completing this form consult your

local Conservation Commission regarding any municipal bylaw or ordinance.

c. City/Town Fee Paid

a. Total Fee Paid



### **Massachusetts Department of Environmental Protection**

Bureau of Resource Protection - Wetlands

### WPA Form 4A - Abbreviated Notice of **Resource Area Delineation**

Ma

Pro	vided by MassDEP:
	MassDEP File Number
	Document Transaction Number
	City/Town

d. Linear Feet Delineated

lass	achusetts	Wetlands Protection Act M.	G.L. C. 131, 940	
				City/Town
3. A	rea(s) De	elineated		
	D landara	1/1-1	15,350 LF	
1.	Bordering	Vegetated Wetland (BVW)	Linear Feet of Boundary De	lineated
2.	Check all r	nethods used to delineate the Bo	ordering Vegetated Wetland	(BVW) boundary:
	a. Ma	assDEP BVW Field Data Form (a	ttached)	
	b. 🗶 Ot	her Methods for Determining the	BVW boundary (attach docu	umentation):
	1. 🔀	50% or more wetland indicator	plants	
	2. 🗌	Saturated/inundated conditions	exist	
	з. 🔲	Groundwater indicators		
	4. 🔲	Direct observation		
	5.	Hydric soil indicators		
	6.	Credible evidence of conditions	prior to disturbance	
3.	Indicate ar	ny other resource area boundarie	s that are delineated:	
1	Wills Bro	ook		1,190 LF
a.	Resource Area			<ul> <li>b. Linear Feet Delineated</li> </ul>

### C. Additional Information

c. Resource Area

Applicants must include the following plans with this Abbreviated Notice of Resource Area Delineation. See instructions for details. Online Users: Attach the Document Transaction Number (provided on your receipt page) for any of the following information you submit to the Department.

- □ ANRAD (Delineation Plans only)
- ☑ USGS or other map of the area (along with a narrative description, if necessary) containing sufficient information for the Conservation Commission and the Department to locate the site. (Electronic filers may omit this item.)
- 3. 🗵 Plans identifying the boundaries of the Bordering Vegetated Wetlands (BVW) (and/or other resource areas, if applicable).
- 🗵 List the titles and final revision dates for all plans and other materials submitted with this Abbreviated Notice of Resource Area Delineation.



### Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

### WPA Form 4A – Abbreviated Notice of Resource Area Delineation

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

### D. Fees

The fees for work proposed under each Abbreviated Notice of Resource Area Delineation must be calculated and submitted to the Conservation Commission and the Department (see Instructions and Wetland Fee Transmittal Form).

1. 🗵 Fee Exempt: No filing fee shall be assessed for projects of any city, town, county, or district of the Commonwealth, federally recognized Indian tribe housing authority, municipal housing authority, or the Massachusetts Bay Transportation Authority.

Applicants must submit the following information (in addition to the attached Wetland Fee Transmittal Form) to confirm fee payment:

2. Municipal Check Number	3. Check date
4. State Check Number	5. Check date
6, Payor name on check: First Name	7. Payor name on check: Last Name



### Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

### WPA Form 4A – Abbreviated Notice of Resource Area Delineation

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Pro	vided by MassDEP:
	MassDEP File Number
	Document Transaction Number
	City/Town

### E. Signatures

I certify under the penalties of perjury that the foregoing Abbreviated Notice of Resource Area Delineation and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

I further certify under penalties of perjury that all abutters were notified of this application, pursuant to the requirements of M.G.L. c. 131, § 40. Notice must be made in writing by hand delivery or certified mail (return receipt requested) to all abutters within 100 feet of the property line of the project location.

I hereby grant permission, to the Agent or member of the Conservation Commission and the Department of Environmental Protection, to enter and inspect the area subject to this Notice at reasonable hours to evaluate the wetland resource boundaries subject to this Notice, and to require the submittal of any data deemed necessary by the Conservation Commission or Department for that evaluation.

I acknowledge that failure to comply with these certification requirements is grounds for the Conservation Commission or the Department to take enforcement action.

Williamster	11-20-62
1. Signature of Applicant	2. Date
3. Signature of Property Owner (if different)	4. Date //- 20 - /2
5. Signature of Representative (it any)	6. Date

### For Conservation Commission:

Two copies of the completed Abbreviated Notice of Resource Area Delineation (Form 4A), including supporting plans and documents; two copies of the ANRAD Wetland Fee Transmittal Form; and the city/town fee payment must be sent to the Conservation Commission by certified mail or hand delivery.

### For MassDEP:

One copy of the completed Abbreviated Notice of Resource Area Delineation (Form 4A), including supporting plans and documents; one copy of the ANRAD Wetland Fee Transmittal Form; and a copy of the state fee payment must be sent to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery. (E-filers may submit these electronically.)

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.

ANRAD Property Owner: Lynnfield Water District Prepared by Gale Associates, Inc. November 21, 2012



### PROPERTY DESCRIPTION

The Lynnfield Center Water District (LWD) property is bounded to the North by the Ipswich River, to the West by Conservation Commission land, to the South by the Sagamore Golf Club, and to the East by land owned by Bostik Findley, Inc. The property consists of twelve (12) parcels owned by the LWD and is approximately 98.5 acres in size. The property does not contain frontage on any right of way, but is located just north of Main Street and south of Elm Street. The property can be accessed from Elm Street at the entrance to the existing LWD pumping station and through the abandoned railroad easement running through the property from east to west.

ANRAD

Property Owner: Lynnfield Water District Prepared by Gale Associates, Inc.

November 21, 2012



### MAP AND PARCEL INFORMATION

Map	Parcel
8	487
8	1279
8	1045
8	1727
8	1822
8	1842
4	647
4	2055
4	1893
4	2771
4	2799
4	2577



November 20, 2012

### Email

John M. Perry, P.E. Gale Associates, Inc. 163 Libbey Parkway Weymouth, MA 02189

Re: Wetland Resource Area Analysis

Lynnfield Recreation Park Land off Main Street Lynnfield, Massachusetts

Dear Mr. Perry:

LEC Environmental Consultants, Inc., (LEC) is pleased to submit this Wetland Resource Area Analysis Report documenting methods and findings associated with a wetland delineation conducted on several lots located on Main Street in Lynnfield, Massachusetts. The purpose of the wetland delineation was to identify the boundaries of Wetland Resource Areas protected under the Massachusetts Wetlands Protection Act (M.G.L. c. 131, s. 40), its implementing Regulations (310 CMR 10.00), and the Lynnfield Environmental Bylaw (Chapter 9) and associated Rules and Regulations.

The subject property boundaries, site topography, and wetland flagging locations are depicted on the Lynnfield Recreation Park Plan, prepared by Eaglebrook Engineering and Surveying, LLC, dated October 25, 2012.

### 1. General Site Description

The site consists of several undeveloped parcels (see Note 4 on the *Plans* for Assessor's reference) totaling approximately 98.5 acres located west off Main Street in the northern portion of Lynnfield, Massachusetts. The parcels are owned by the Town of Lynnfield Center Water District, with the exception of Map 9, Lot 192 and Map 5, Lot 1584 which are privately owned. Map 9, Lot 192 contains frontage on Main Street and does not contain any Wetland Resource Areas.

The site includes extensive forested uplands and forested wetlands with a network of cart paths and trails traversing the site. ATV use of the cart paths and trails is evident. An abandoned railroad easement extends across the northern portion of the property. The northern limits of the site extend to the centerline of the Ipswich River while Will's Brook flows across the southern portion of the site. Each of these rivers is identified as a perennial stream.

Property to the east consists of undeveloped uplands and scattered single-family dwellings along Main Street, while an industrial building exists to the northeast. Property to the west is primarily town-owned undeveloped forested wetlands and uplands while property to the south/southeast is occupied by a golf course.

www.lecenvironmental.com

[LEC File #: GAI\12-224.01]

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The forested upland is defined by a mixed canopy of red oak (Quercus rubra), white oak (Quercus alba), eastern white pine (Pinus strobus), and red maple (Acer rubrum), with scattered individuals of sassafras (Sassafras albidum) and swamp tupelo (Nyssa sylvatica). The upland understory is comprised of witch hazel (Hammamelis virginiana), sweet pepperbush (Clethra alnifolia), highbush blueberry (Vaccinium corymbosum), dangleberry (Gaylussacia frondosa), black huckleberry (Gaylussacia baccata), and arrowwood (Viburnum dentatum), with entanglements of greenbrier (Smilax rotundifolia). Groundcover consists of tree clubmoss (Lycopotium obscurum), teaberry (Gaultheria procumbens), and hay-scented fern (Dennstaedtia punctilobula).

According to the Essex County Soil Survey, the site includes areas mapped as Whitman loam, extremely stony; Scarboro mucky loamy fine sand; Windsor loamy sand; Paxton fine sandy loam, very stony; Woodbridge fine sandy loam, very stony; and Poquonock loamy sand, very stony. Overall the site is defined by glacial till with stony soils and scattered boulders at the surface.

### 1.1 Floodplain Designation

According to FEMA Flood Insurance Rate Map for the Town of Lynnfield dated July 3, 2012 (Community Punel 25009C0383F), the site contains extensive areas mapped as Zone A, Areas subject to inundation by the 1% annual chance flood. The extent of Zone A is roughly coincident with the extent of BVW on the property. The upland portions of the property are mapped as Zone X, Areas determined to be outside the 100-year flood plain.

### 1.2 Massachusetts Natural Heritage & Endangered Species Program Designation

According to the 13<sup>th</sup> edition of the Massachusetts Natural Heritage Atlas (valid from October 1, 2008) published by the Natural Heritage & Endangered Species Program (NHESP), the property is not located within an *Estimated or Priority Habitat for Rure Species*. According to the MassGIS datalayers, there are no certified or potential vernal pools on the site.

### 2. Wetland Delineation Methodology

The extent of the Bordering Vegetated Wetlands (BVW) was determined through observations of the existing plant communities, using the "fifty percent criteria" to determine dominance of wetland/upland vegetation, the interpretation of soil characteristics, and other indicators of wetland hydrology in accordance with the criteria enumerated within 310 CMR 10.55 (2), the Handbook prepared by the Massachusetts Department of Environmental Protection, entitled *Delineating Bordering Vegetated Wetlands Under the Massachusetts Wetlands Protection Act* (March 1995), as well as the *Field Indicators for Identifying Hydric Soils in New England* (April 2004). The boundaries are demarcated in the field with blaze orange surveyor's flagging tape embossed with the words "LEC Resource Area Boundary" in bold, black print and numbered A1-A53, B1-B193, C1-C32, D1-D63, and E1-E39.

In addition, LEC demarcated the Mean Annual High Water (MAHW) to the Ipswich River and Will's Brook in accordance with 310 CMR 10.58 (2) (a) (2). MAHW was demarcated with blue flagging tape numbered MAHW 1-4 along the Ipswich River and MAHW 1A-27A/1B-27B along Will's Brook.



### 3. Wetland Resource Area Descriptions

Wetland Resource Areas located on or adjacent to the property include BVW, Riverfront Area, and Bank. The site may also contain Bordering Land Subject to Flooding (BLSF) as described below. A brief description of each Wetland Resource Area follows.

### 3.1 BVW

BVW is defined at 310 CMR 10.55 (2) (a) as freshwater wetlands that horder on creeks, rivers, streams, ponds and lakes. The types of freshwater wetlands are wet meadows, marshes, swamps and bogs. Bordering Vegetated Wetlands are areas where the soils are saturated and/or inundated such that they support a predominance of wetland indicator plants.

The property contains extensive forested BVW areas associated with a series of intermittent streams as well as the perennial streams. The BVW canopy typically consists of red maple (Acer rubrum) and eastern white pine (Pinus strobus), with scattered individuals of red oak (Quercus rubra), white oak (Quercus alba), and swamp tupelo (Nyssa sylvatica). The understory is variably dense, with some very dense areas dominated by sweet pepperbush, while other areas are more open allowing conspicuous patches of cinnamon fern (Osmunda cinnamomeo), goldthread (Coptis trifolia), and sphagnum moss (Sphagnum spp.) to thrive. Other species observed in the BVW include highbush blueberry (Vaccinium corymbosum), arrowwood (Viburnum dentatum), swamp azalea (Rhododendron viscosum), witch hazel (Hamamelis virginiana), royal fern (Osmunda regalis), and sensitive fern (Onoclea sensibilis).

The C-series wetland flagging is a smaller, seemingly isolated system separated from the larger BVW areas. The wetland does contain evidence of flow that may be considered an intermittent stream; however, the overland does not appear to have created an incised channel and thus categorizing the area as BVW (as we have) bordering on an intermittent stream may be disputed. In any case, the Lynnfield *Bylaw* protects isolated wetlands so the C-series wetland is a protected Wetland Resource Area under that statute regardless of whether or not it borders on a stream.

### 3.2 Riverfront Area

According to 310 CMR 10.58(2)(a)(3), Riverfront Area is defined as the area of land between a river's mean annual high-water line measured horizontally outward from the river and a parallel line located 200 feet away.

The Mean Annual High-Water (MAHW) Line to perennial streams is delineated in the field to establish the limits of Riverfront Area. MAHW was delineated in accordance with definition in 310 CMR 10.58 (2)(a)2. - the line that is apparent from visible markings or changes in the character of soils or vegetation due to the prolonged presence of water and that distinguishes between predominantly aquatic and predominantly terrestrial land. Field indicators of bankfull conditions shall be used to determine the mean annual highwater line. Bankfull indicators include but are not limited to: changes in slope, changes in vegetation, stain lines, top of pointbars, changes in bank materials, or bank undercuts.

Riverfront Area is associated with a small segment of the Ipswich River on the northern portion of the site and with Will's Brook on the southern portion of the property. In each case, the 200-foot Riverfront Area comprises forested uplands and forested wetlands.



### 3.3 Bank

Bank is defined at 310 CMR 10.54(2)(a) as the portion of land surface which normally abuts and confines a water body. The upper boundary of a bank is the first observable break in the slope or the mean annual flood level, whichever is lower. The lower boundary of a bank is the mean annual low flow level.

Bank is associated with the perennial streams described above and is coincident with the delineated MAHW line. Additional Bank is associated with numerous intermittent streams contained within the BVW.

### 3.4 Bordering Land Subject to Flooding

According to 310 CMR 10.57(2)(a)(1), BLSF is an area with low, flat topography adjacent to and inundated by flood waters rising from creeks, rivers, streams, ponds or lakes. It extends from the banks of these waterways and water bodies; where a bordering vegetated wetland occurs, it extends from said wetland.

BLSF exists in those portions of the property where the 100-year flood plain extends beyond the BVW boundary (i.e. into the upland). Further review of the actual extent of the flood plain would be necessary to determine the extent of BLSF on the property.

### 4. Regulatory Implications

The site contains Bordering Vegetated Wetlands, Bank, and Riverfront Area; Wetland Resource Areas protected under the Massachusetts Wetlands Protection Act (M.G.L. c. 131, s. 40), its implementing Regulations (310 CMR 10.00), and the Lynnfield Environmental Bylaw (Chapter 9) and associated Rules and Regulations. As described above, the site may also contain BLSF. Any proposed activity or alteration within the above-referenced Wetland Resource Areas or their Buffer Zones will require filing the necessary permit applications with the Lynnfield Conservation Commission and the Massachusetts Department of Environmental Protection (DEP).

Sincerely,

LEC Environmental Consultants, Inc.

Mark L. Manganello

Assistant Director of Ecological Services

Mark L. Manjanello (ss.)

## - Approximate property lines

Golfse

(19,24/2012)		LYNNFIELD					Page 1 of 1
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ParcellD	Location	Owner	Co-Owner	Mailing Address	City	Stat	State Zip
0002 0000 2555	LOWELL ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	ΜĀ	01940
0004 0000 0498	REAR MAIN ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	MΑ	01940
0004 0000 0647	REAR MAIN ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	MΑ	01940
0004 0000 1091	REAR MAIN ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	Ψ	01940
0004 0000 1893	REAR MAIN ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	MA	01940
0004 0000 2055	6 REAR MAIN ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	¥	01940
0004 0000 2399	_	LYNNFIELD TOWN OF	CONSERVATION COMMISSIV	TOWN HALL	LYNNFIELD	MA	01940
0004 0000 2577	REAR LOWELL ST	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	MA	01940
0004 0000 2771	REAR MAIN ST BMRR SOUTH	LYNNFIELD TOWN OF	CONSERVATION COMMISSI	TOWN HALL	LYNNFIELD	MA	01940
0004 0000 2799	REAR MAIN ST BMRR SOUTH	LYNNFIELD TOWN OF	CONSERVATION COMMISSI	$\vdash$	LYNNFIELD	¥	01940
0005 0000 2100	REAR MAIN ST	BOSTIK FINDLEY, INC.		11320 WATERTOWN PLANK	WAUWATOSA	<u>×</u>	53226-3413
0005 0000 0117	REAR MAIN ST	BOSTIK FINDLEY, INC.		11320 WATERTOWN PLANK	WAUWATOSA	3	53226-3413
0005 0000 3517	REAR MAIN ST	BOSTIK FINDLEY, INC		11320 WATERTOWN PLANK		₹	53226-3413
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0008 0000 0014 0008 0000 1045	MIDDLETON HILL	LYNNFIELD CTR WATER DIST		83 PHILLIPS ROAD	LYNNFIELD	MA	01940
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0012 0000 0466	1282 MAIN ST	SAGAMORE SPRING REALTY TRU LUFF, LUFF & THOMPSON T	J LUFF, LUFF & THOMPSONT	1282	LYNNFIELD	MA	01940
	Town of North Reading			235 North Street	N. Reading	MA	01864-1258
		Estate		10 Park Plaza	Boston	Ψ¥	02116

End of Report

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LYNNFIELD CTR WATER DIST	#P 4. 800	BOSTIK FINDLEY, INC.		LYNNFIELD CTR WATER DIST	
83 PHILLIPS ROAD		11320 WATERTOWN PLANK RD		83 PHILLIPS ROAD	
LYNNFIELD, MA 01940		ATTN: NANCY BAKER		LYNNFIELD, MA 01940	
		WAUWATOSA, WI 53226-3413			
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LYNNFIELD, MA 01940		ATTN: NANCY BAKER		LIMMIELD, MM 01840	
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LYNNFIELD, MA 01940		LYNNFIELD, IMA 01340			
	0004 0000 0000	REAR MAIN ST	0008 0000 1279		
REAR MAIN ST	0004 0000 2399	REAR MAIN 31	LUC: 903	-	
	LUC: 903	LYNNFIELD TOWN OF		MBTA Director of R	eal Estate
LYNNFIELD TOWN OF		CONSERVATION COMMISSION		10 Park Plaza	
CONSERVATION COMMISSION		TOWN HALL		Boston, MA 02116	8.0
TOWN HALL		LYNNFIELD, MA 01940			
LYNNFIELD, MA 01940	0004 0000 2577	1462 REAR MAIN ST	0008 0000 1325		
REAR LOWELL ST	0004 0000 2577 LUC: 903	1402 REAR MAIN OF	LUC: 601		
DANIEL BOTH WATER DIST	200. 903	RICHARDSON GREEN			
LYNNFIELD CTR WATER DIST		PO BOX 499			
83 PHILLIPS ROAD LYNNFIELD, MA 01940		MIDDLETON, MA 01949			
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REAR MAIN ST BMRR SOUTH	0004 0000 2771	1466 MAIN ST	0008 0000 1431		
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CONSERVATION COMMISSION		BRETTELL NORMAN F			
		1466 MAIN ST			
TOWN HALL LYNNFIELD, MA 01940		LYNNFIELD, MA 01940			
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LYNNFIELD TOWN OF		83 PHILLIPS ROAD			
CONSERVATION COMMISSION		LYNNFIELD, MA 01940			
TOWN HALL					
LYNNFIELD, MA 01940					

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940

Ph:781-334-9450 Fax:781-334-9419

LYNNFIELD BOARD OF ASSESSORS

To be submitted to (check one)
Board of Appeals Board of Health Conservation Commission Planning Board Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynn field Center Water District
Address of Property:
Assessor's Map # 8 Parcel # 1045
Name of Applicant (print) Town OF YNNFIELD
Applicant Address 55 SUMMER ST
(No. & Street)  LYNNFIELD, MA
Applicant's Telephone 781-3342128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
Certified by: Name Hardsel  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940 Ph:781-334-9450 Fax:781-334-9419

LYNNFIELD BOARD OF ASSESSORS

To be submitted to (check one)
Board of Appeals Board of Health Conservation Commission Planning Board Other
If this form is not complete it will be returned.
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Property owner of record: Lynnfield Center Water District
Address of Property:
Assessor's Map # 8 Parcel # 1727
Name of Applicant (print) Town OF UY NUFIELD
Applicant Address SS SUMMER ST
(No. & Street)  LYNNFIELD MA
(City/Town & State) Applicant's Telephone 781-33 4-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYSAssessors Use Only
Certified by: Marie Hardrer  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940

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LYNNFIELD BOARD OF ASSESSORS

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Property owner of record: Lynnfield Center Water District
Address of Property:
Assessor's Map # 8 Parcel # 1279
Name of Applicant (print) TOWN OF LYNNFIELD
Applicant Address SS SUMMERST.  (No. & Street)
Applicant's Telephone 781-334-312-8  (City/Town & State)
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYSAssessors Use Only
Certified by: Marie Hardrey  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS

55 Summer Street
Lynnfield, MA 01940
Ph:781-334-9450 Fax:781-334-9419

LYNNFIELD BOARD OF ASSESSORS

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Board of Appeals Board of Health Conservation Commission Planning Board Other
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Property owner of record: Lynnfidd Center Water District
Address of Property:
Assessor's Map # 8 Parcel # 1842
Name of Applicant (print) TOWN OF LYNNHELD
Applicant Address SS SUMMED ST  LYNNFIELD (No. & Street)  (City/Town & State)
LYNNFIED, (No. & Street)
Applicant's Telephone 781-334-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
Assessors Use Only
Certified by: Marie Hardrer
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940

Ph:781-334-9450 Fax:781-334-9419

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Property owner of record: Lynnfield Center Water District
Address of Property:
Assessor's Map # 8 Parcel # 1822
Name of Applicant (print) TOWN OF LYNNFIELD
Applicant Address 55 SUMMER ST
(No. & Street)
(City/Town & State) Applicant's Telephone 7 81 -337-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING
DAYSAssessors Use Only
Certified by: Marie Hardner  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940 Ph:781-334-9450 Fax:781-334-9419

LYNNFIELD BOARD OF ASSESSORS

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Board of Appeals Board of Health Conservation Commission Planning Board Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynn field Center Water Defrict
Address of Property:
Assessor's Map # 8 Parcel # 487
Name of Applicant (print) TOUN OF LTWNFIELD
Applicant Address 355 SUMMER ST. (No. & Street)
(No. & Street)  LYNNFIELD, MA  (City/Town & State)  Applicant's Telephone 781-337-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
Certified by: Marie Hardner  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940

Ph:781-334-9450 Fax:78

A 01940 LYNNFIELD BOARD OF ASSESSORS

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Board of Appeals Board of Health Conservation Commission Planning Board Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynnfield Center Water Dotnet
Address of Property:
Assessor's Map # 4 Parcel # 2799
Name of Applicant (print) TOWN OF LYNNFIELD
Applicant Address SS SUMMER ST
Applicant Address (No. & Street)  (No. & Street)  (City/Town & State)
(City/Town & State) Applicant's Telephone 781-734-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS  Assessors Use Only
Certified by: Marie Hardner  Date: 9/24/12
Date: 9/24/12

### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

To be submitted to (check one)
Board of Appeals Board of Health Conservation Commission Planning Board Other
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Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynnfield Center Water District  Address of Property: N/A
Assessor's Map # 4 Parcel # 2055
Name of Applicant (print) Town OF LYNNFIELD
Applicant Address SS SUMMER ST.  (No. & Street)
(City/Town & State) Applicant's Telephone 781-334-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
Certified by: Marie Gardner
Date: 9/24/12

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LYNNFIELD BOARD OF ASSESSORS

### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

To be submitted to (check one)

Board of Appeals
Board of Health
Conservation Commission
Planning Board
Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lymnfield Center Water District
Address of Property:
4 447
Assessor's Map # 4 Parcel # 647
Name of Applicant (print) TOWN OF LYNNFIED
Applicant Address 55 SUMMER ST.
Applicant Address (No. & Street)  Lynnfeld, MA  (City/Town & State)
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(City/Town & State) Applicant's Telephone 781-334-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
DAYS  Assessors Use Only
Certified by: Marie Hardrer  Date: 9/24/12
Date: 9/24/12

### TOWN OF LYNNFIELD BOARD OF ASSESSORS 55 Summer Street Lynnfield, MA 01940

Ph:781-334-9450 Fax:781-334-9419

LYNNFIELD BOARD OF ASSESSORS

### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

To be submitted to (check one)
Board of Appeals Board of Health Conservation Commission Planning Board Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynnhedd Center Water District
Address of Property:
Assessor's Map # 4 Parcel # 2771
Name of Applicant (print) TOWN OF LYNNFIELD
Applicant Address SS SUMMER ST (No. & Street)
(City/Town & State) Applicant's Telephone 781-334-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYSAssessors Use Only
Certified by: Marie Hardner  Date: 9/24/12
Date: 9/24/12

LYNNFIELD BOARD OF ASSESSORS

### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

To be submitted to (check one)

Board of Appeals Board of Health
✓ Conservation Commission
Planning Board
Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
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Property owner of record: Lynnfield Center Water Detnet
Address of Property:
Assessor's Map # 4 Parcel # 2577
Name of Applicant (print) TOWN OF LYNNFIELD
Applicant Address SS SUMMER ST.
(No. & Street)  LYNNFIELD, MA
(City/Town & State)
Applicant's Telephone 781-334-2128
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CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS
Accessors Use Only
Certified by: Marie Hardner  Date: 9/24/12
Date: 9/24/12



### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

Board of Appeals
Board of Health

To be submitted to (check one)

Conservation Commission
Planning Board Other
If this form is not complete it will be returned.
Fee: \$5.00 for first five pages \$1.00 each consecutive page
Property owner of record: Lynnfield Center Water District
Address of Property: NA
Assessor's Map # Parcel # 1893
Name of Applicant (print) TOWN OF LYNNFIELD
CE CLIMMER ST
Applicant Address (No. & Street)  LYNNFIELD, MA  (City/Town & State)
(City/Town & State) Applicant's Telephone 781-334-3128
CERTIFIED LIST WILL BE PROVIDED WITHIN SEVEN TO TEN WORKING DAYS  Assessors Use Only
Certified by: Marie Hardrer  Date: 9124/12
Date: 9/24/12

### REQUEST FOR CERTIFICATION OF ABUTTER'S LIST

To be submitted to (check one)	
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Board of AppealsBoard of Health	W. 12444694 CVV
Conservation Commission Planning Board	NUA 5 & 5015
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If this form is not complete it will be returned.	
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	5/ 9)
Property owner of record:	)2
Address of Property: Rear Main St	9 
Assessor's Map # 5 Parcel # 1584	0 9
Name of Applicant (print)	2 
Applicant Address Town of Lynnfiel (No. & Street)	21
Applicant's Telephone(City/Town & State)	
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	LUC: 903	
LYNNFIELD CTR WATER DIST		BOSTIK FINDLEY, INC
83 PHILLIPS ROAD		11320 WATERTOWN PLANK RD
LYNNFIELD MA 01940	6.	ATTN: NANCY BAKER WAUWATOSA WI 53226-3413
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ParcellD	Location	Owner	Co-Owner	Mailing Address	City	Stat	State Zip
0005 0000 1584	REAR MAIN ST	BOSTIK FINDLEY, INC		11320 WATERTOWN PLANK WAUWATOSA	WAUWATOSA	3	53226-3413
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# End of Report

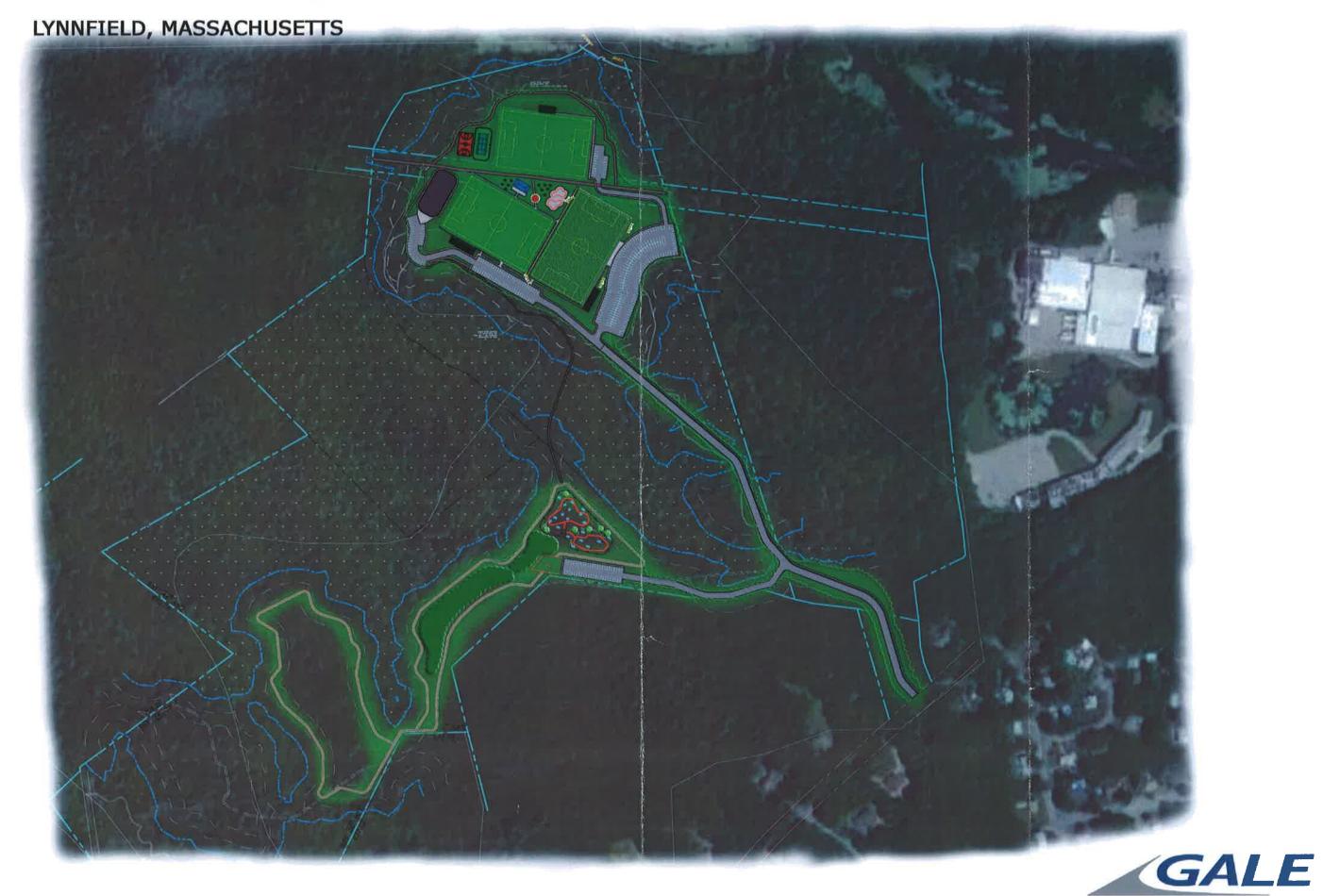
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LYNNFIELD MA 01940	
100	LUC: 101
ROY JOSEPH G JR ROY EDITH A 1489 MAIN ST	er T

LYNNFIELD MA 01940

## ${\bf Enclosure} \ {\bf 6}$ ${\bf Conceptual} \ {\bf Schematics-Recreation} \ {\bf Complex}$

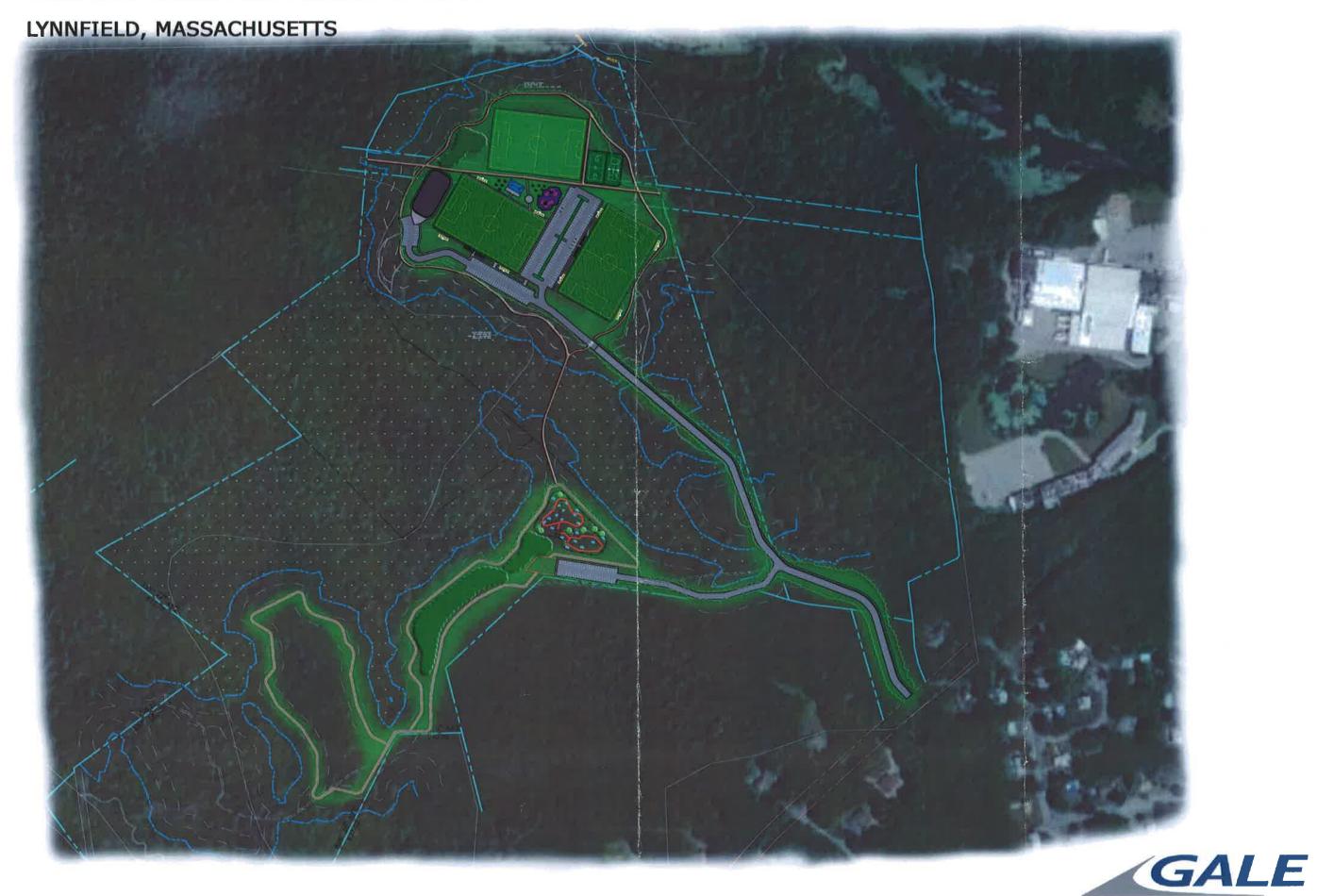
### Development Option 1

### LYNNFIELD RECREATION PROJECT OPTION 1



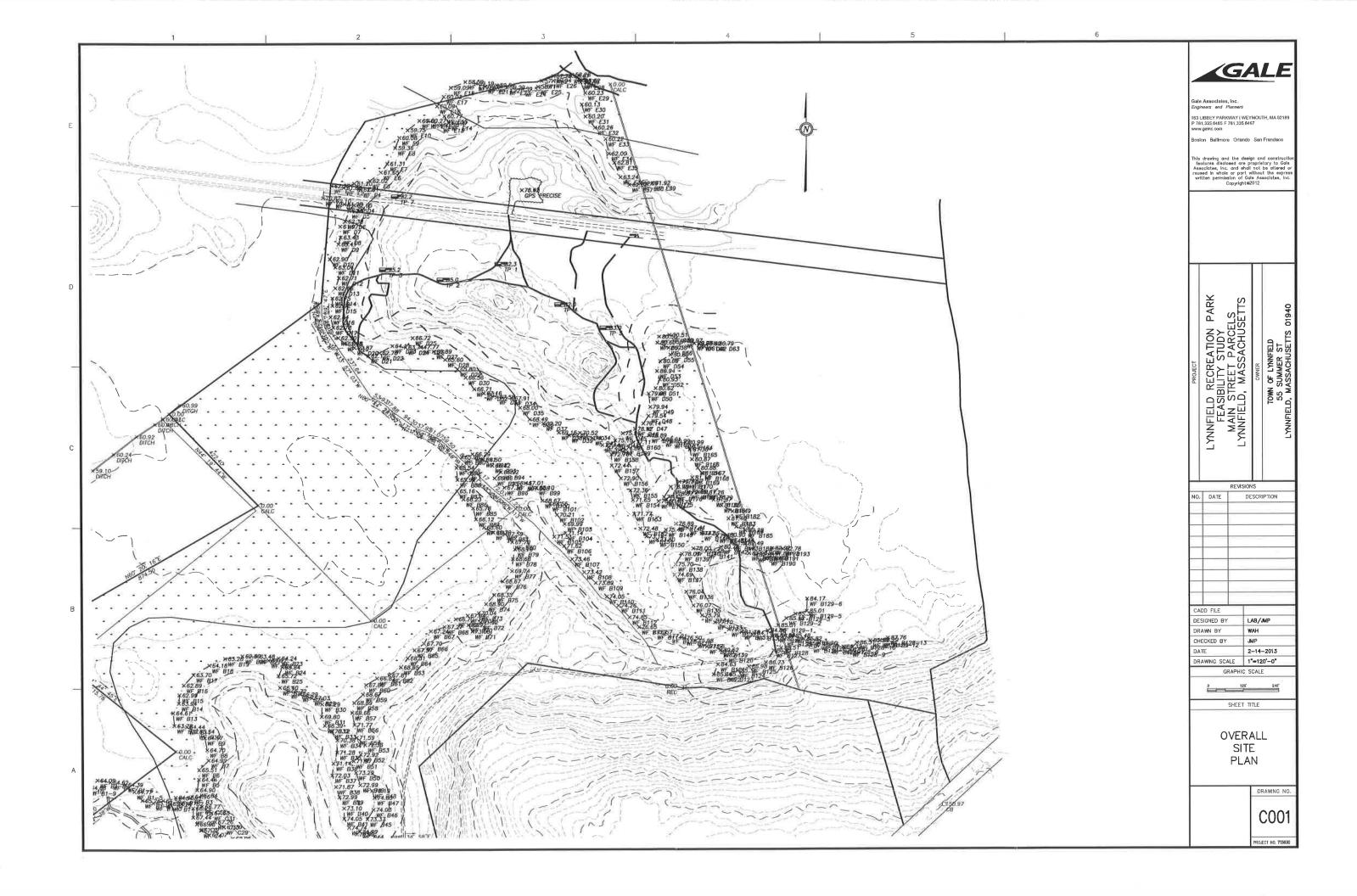
Development Option 2

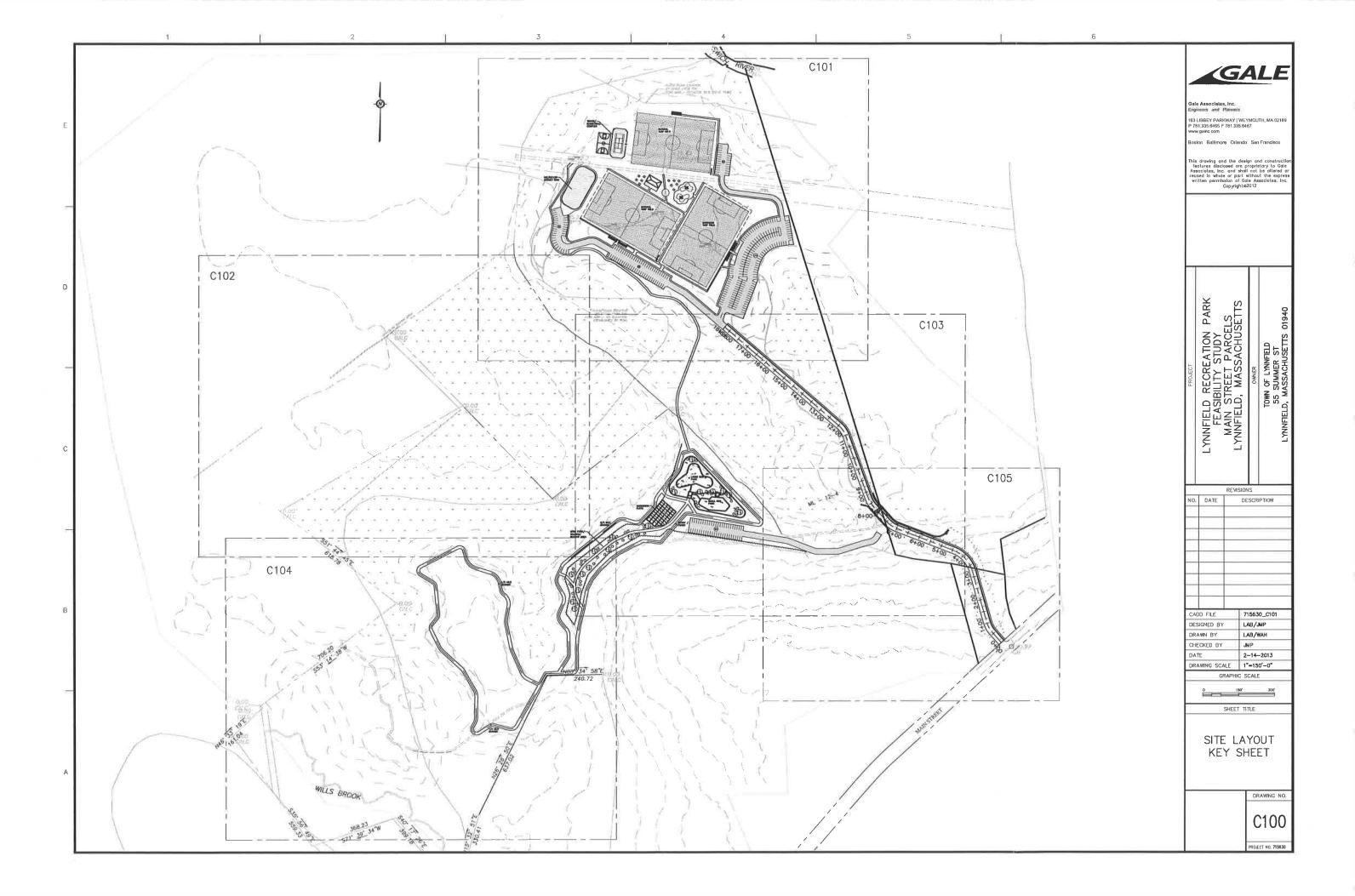
### LYNNFIELD RECREATION PROJECT OPTION 2

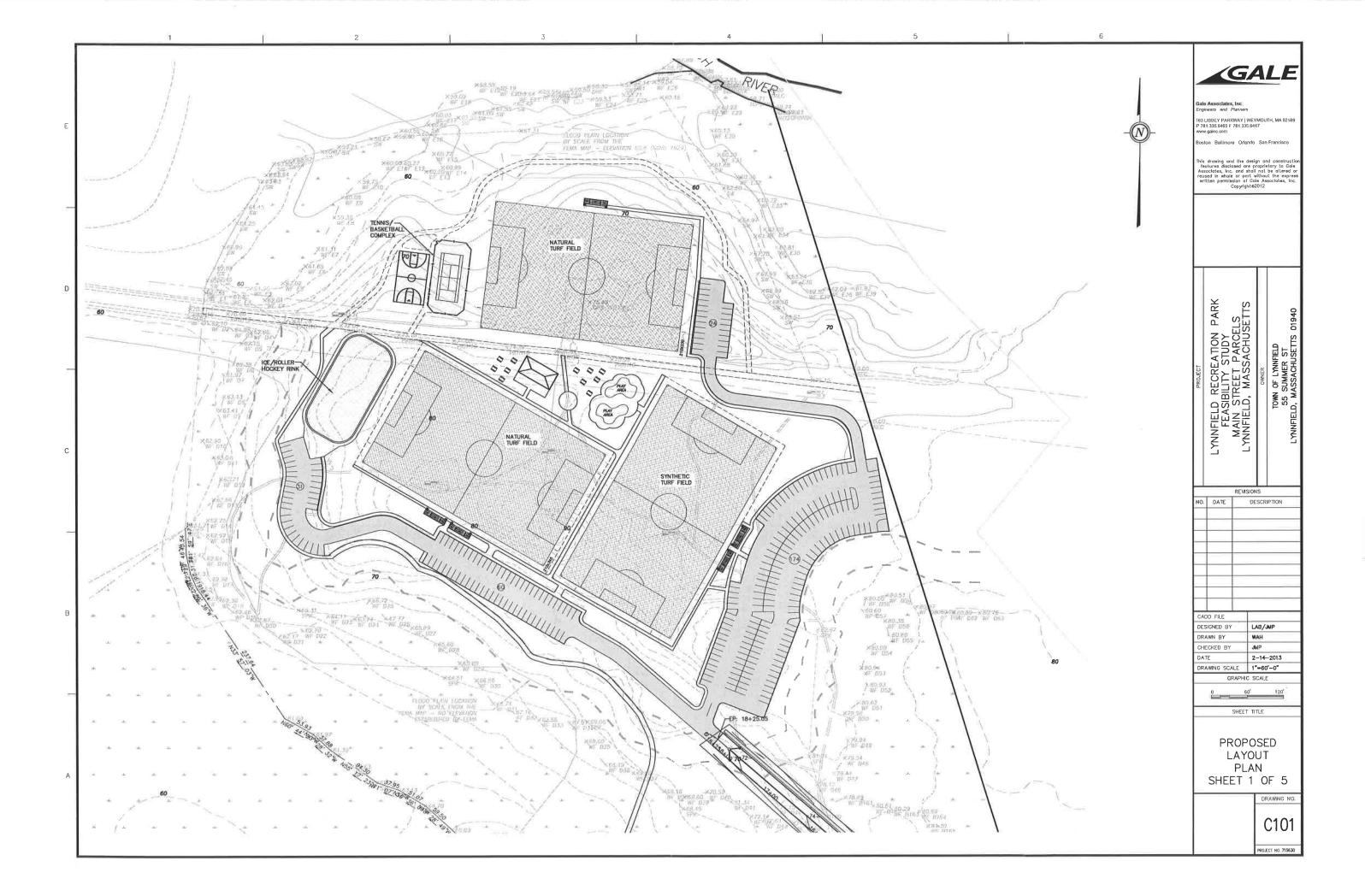


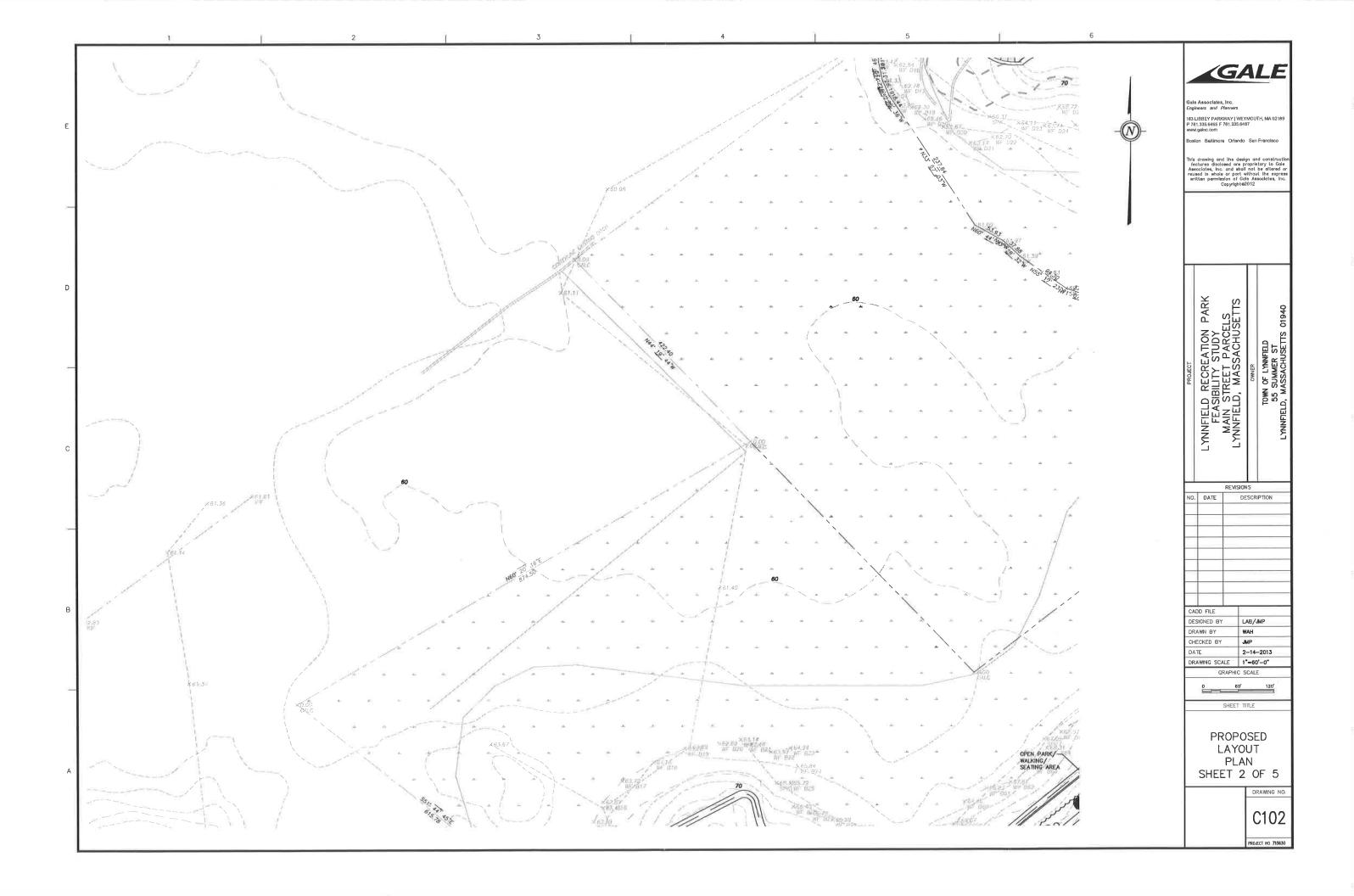
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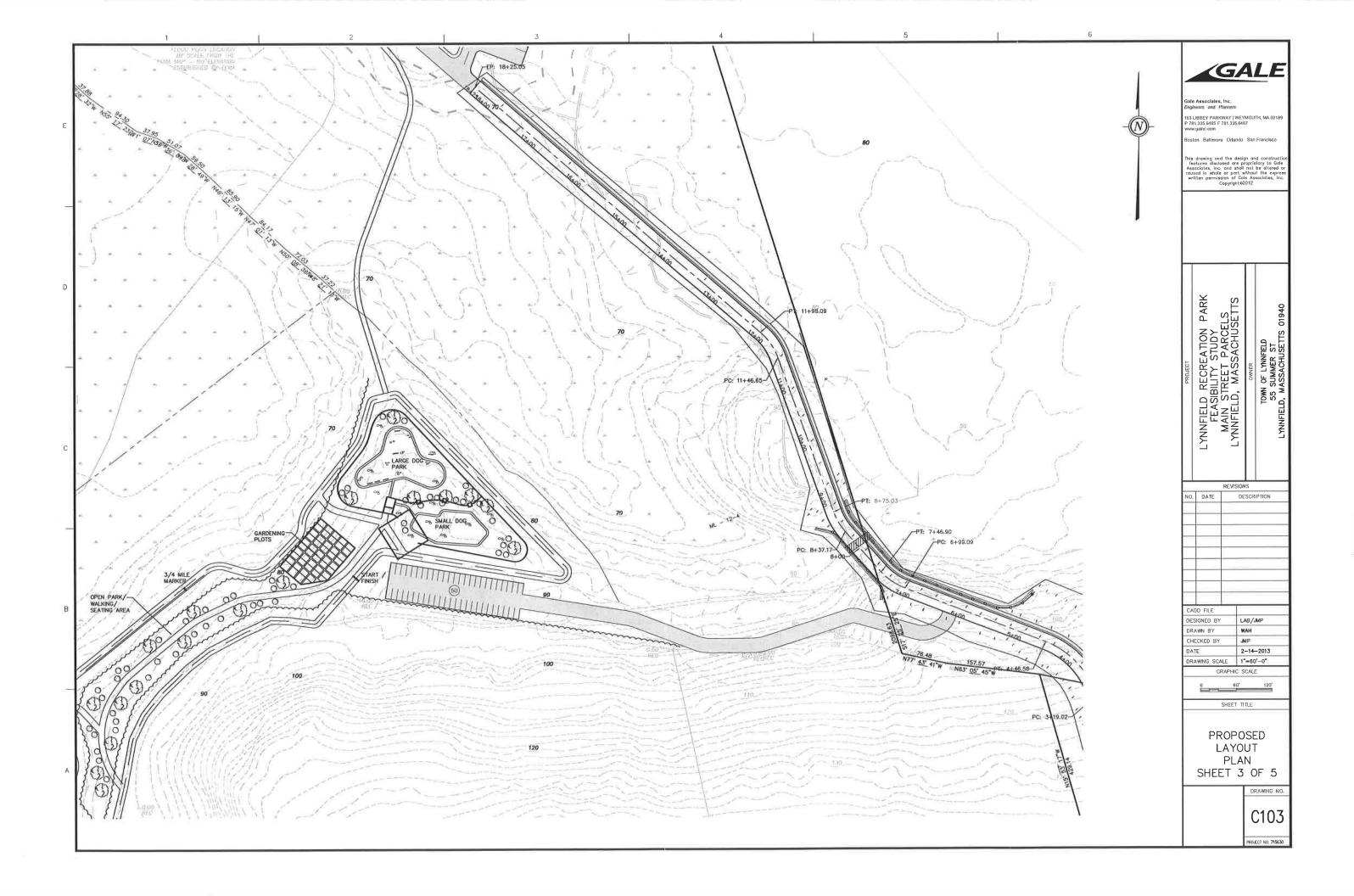
### Enclosure 7 Access Roadway Plans

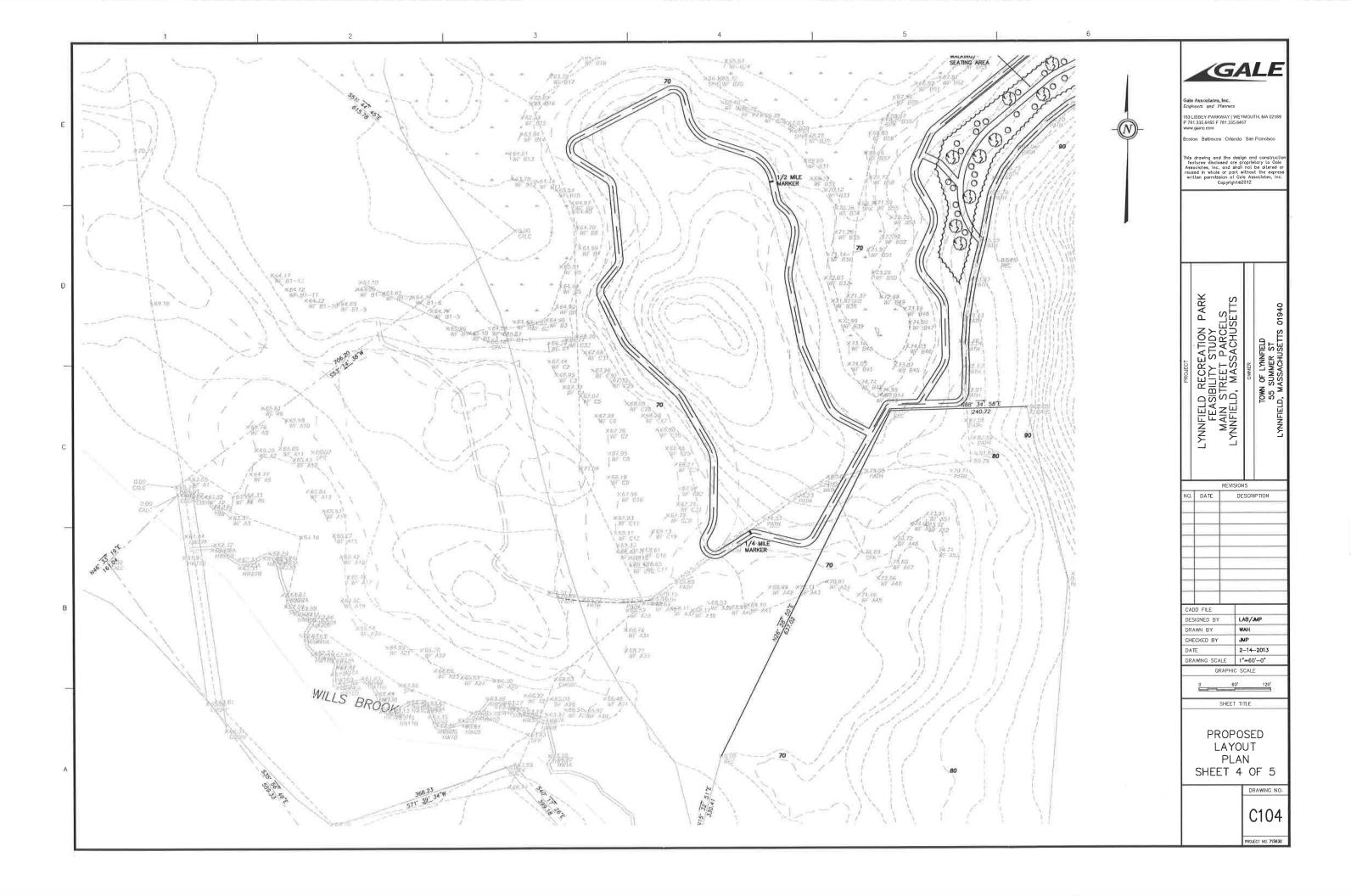


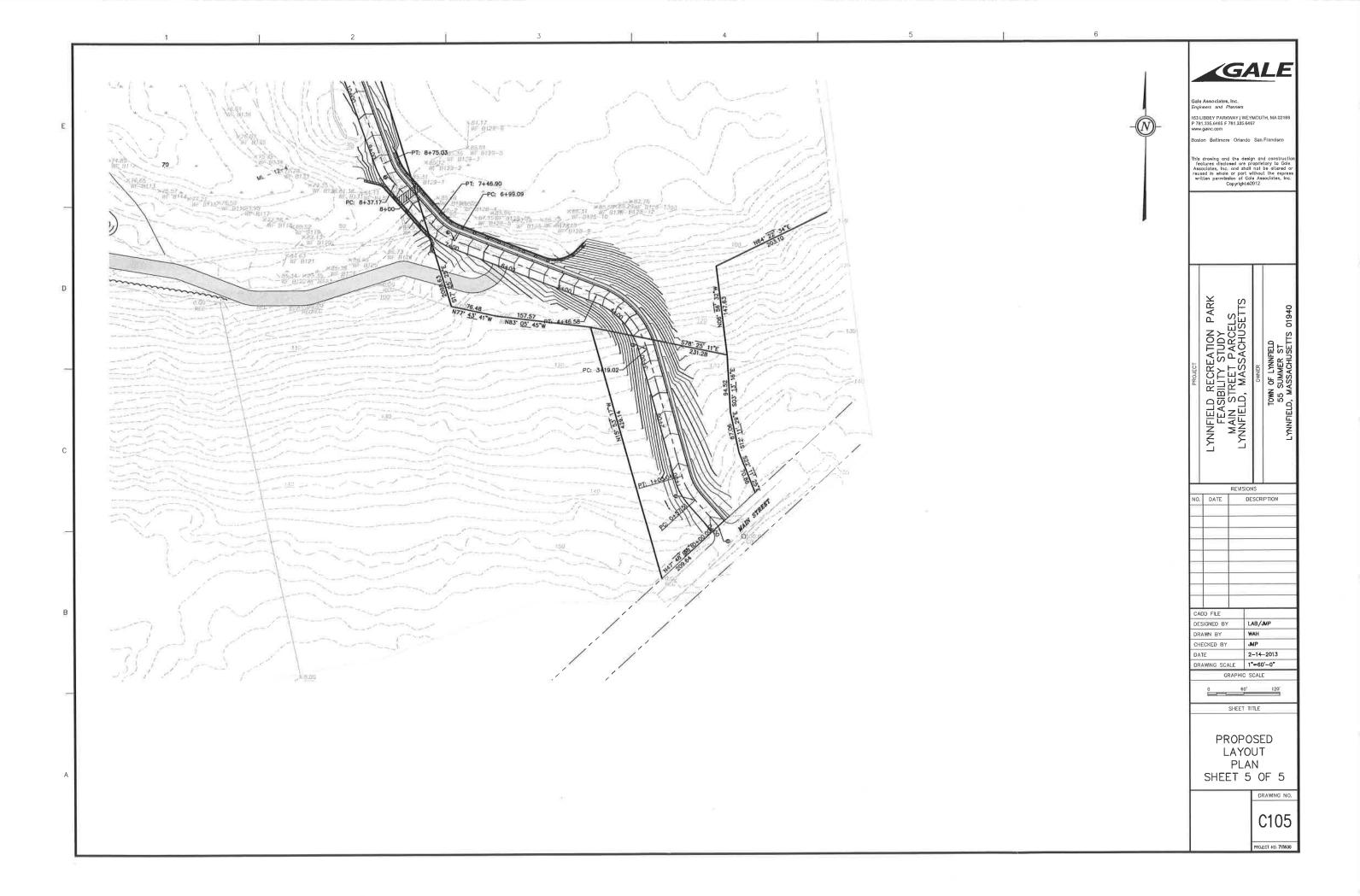


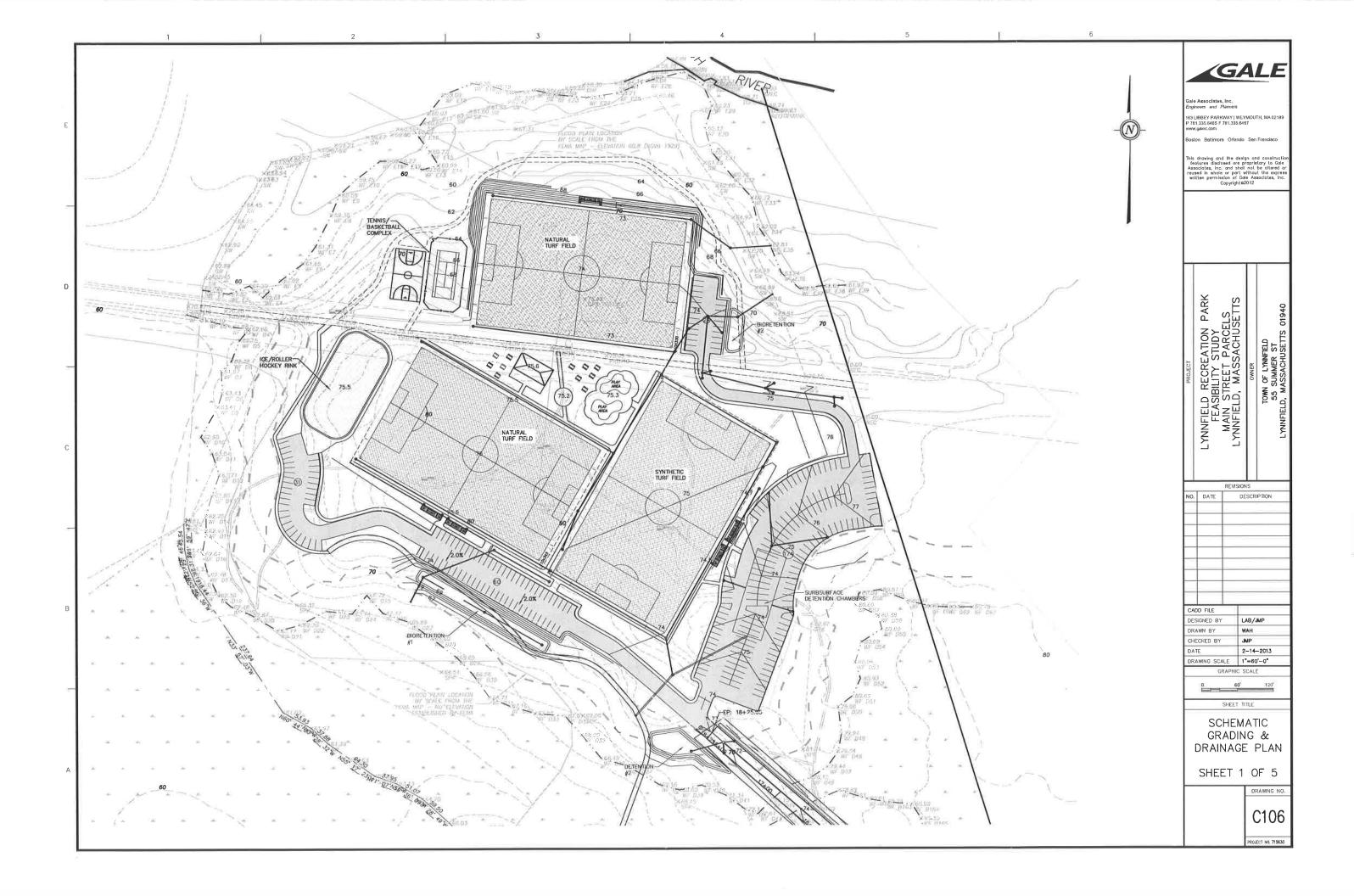


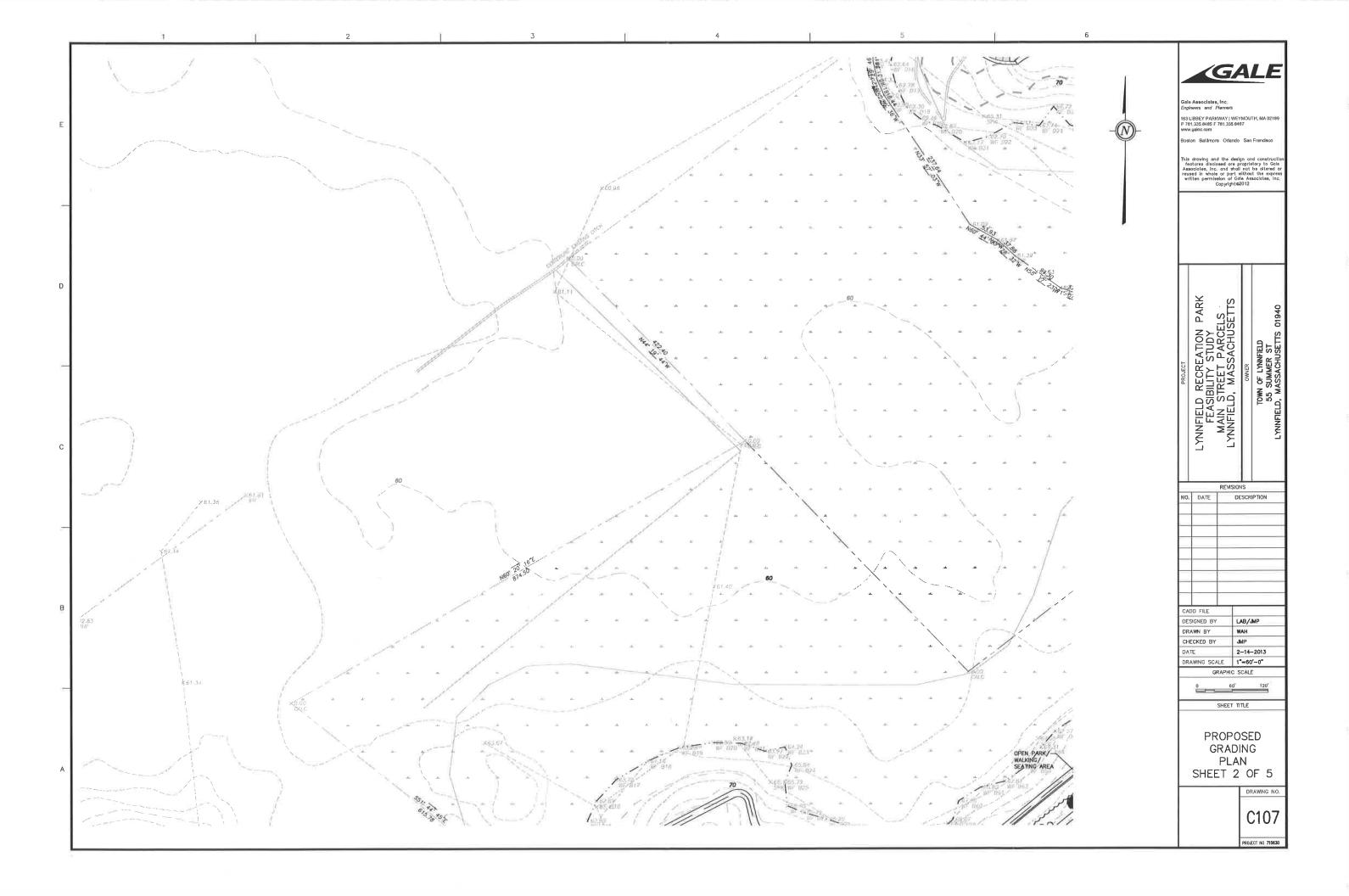


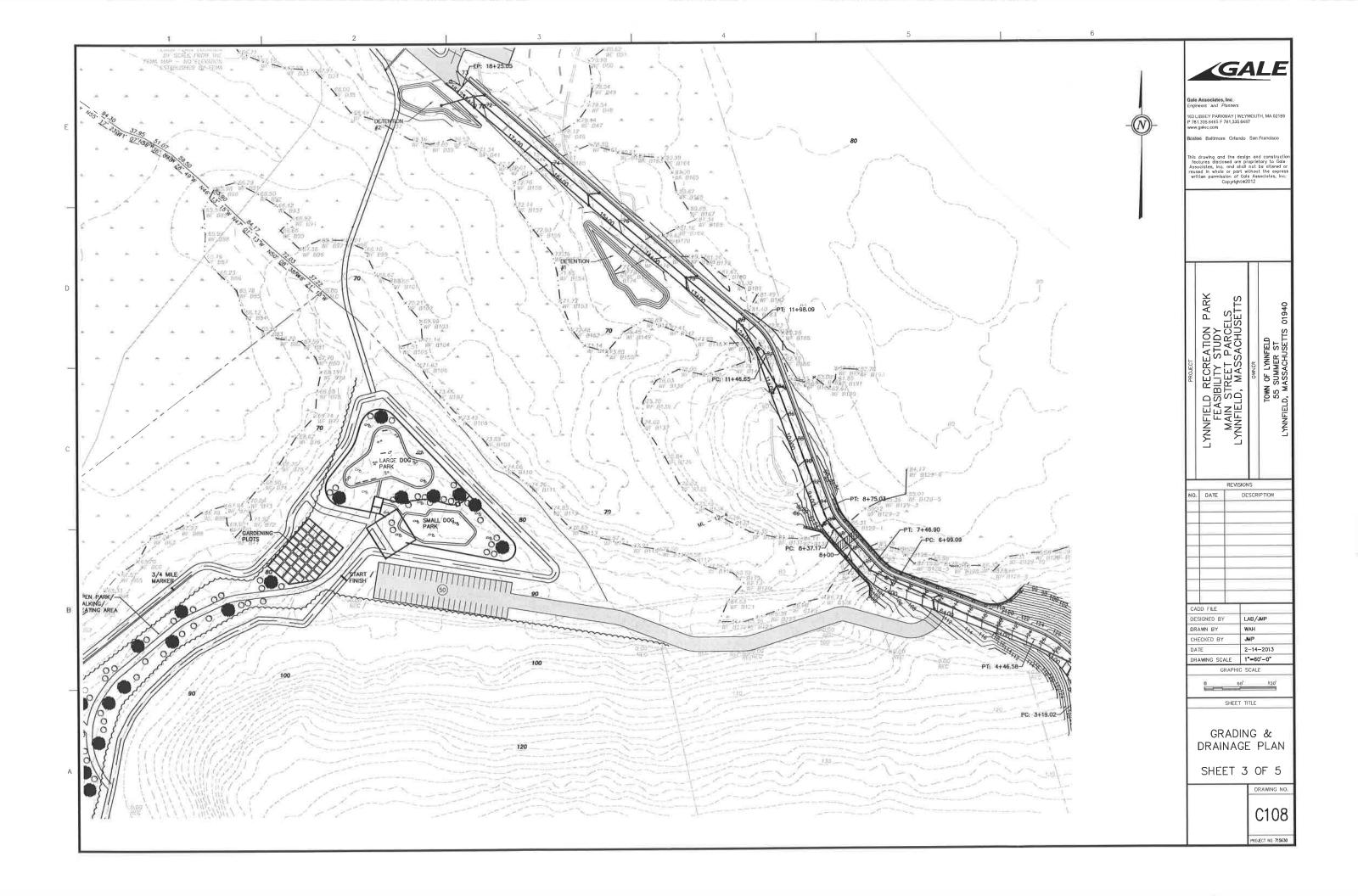


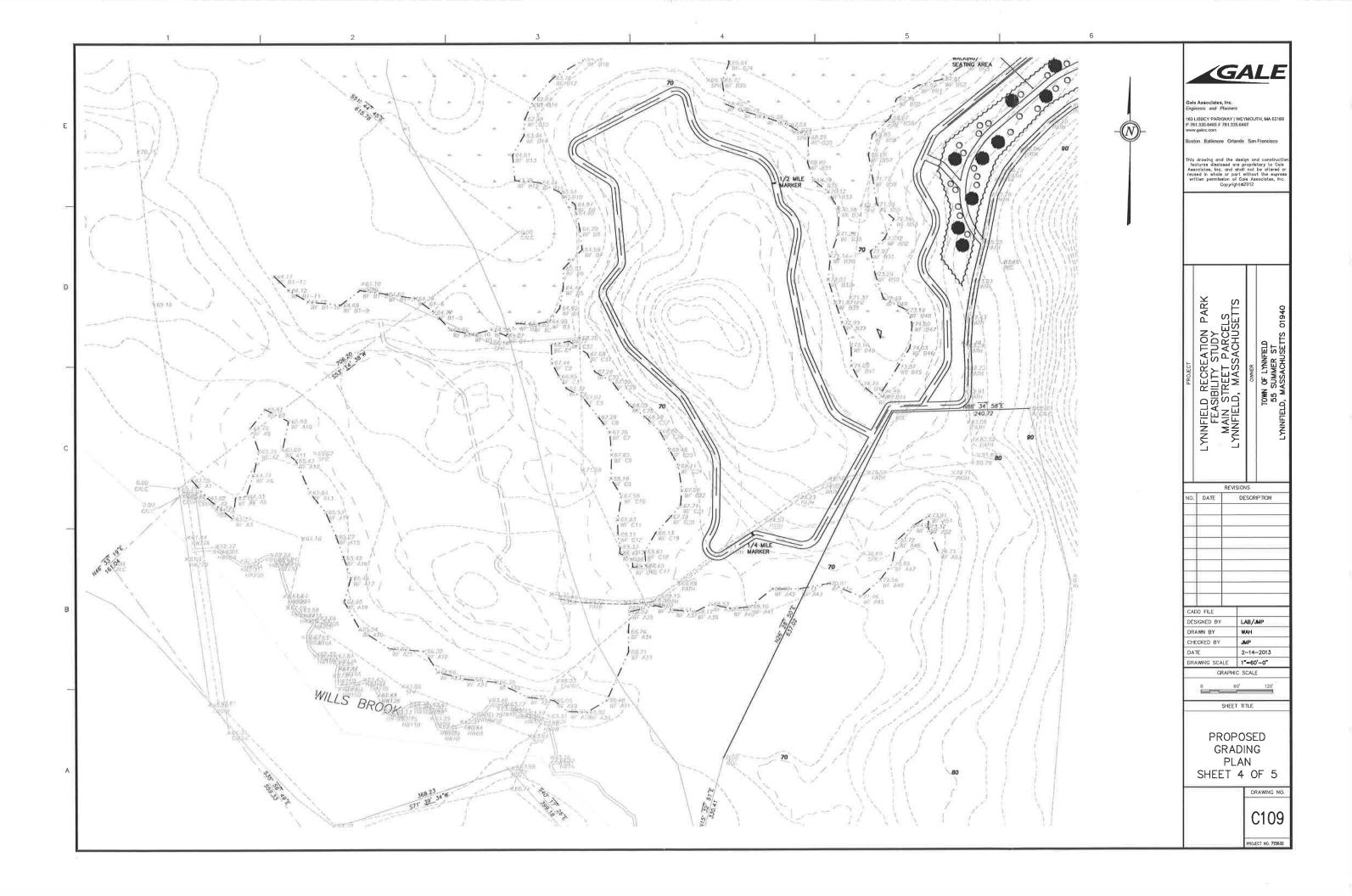


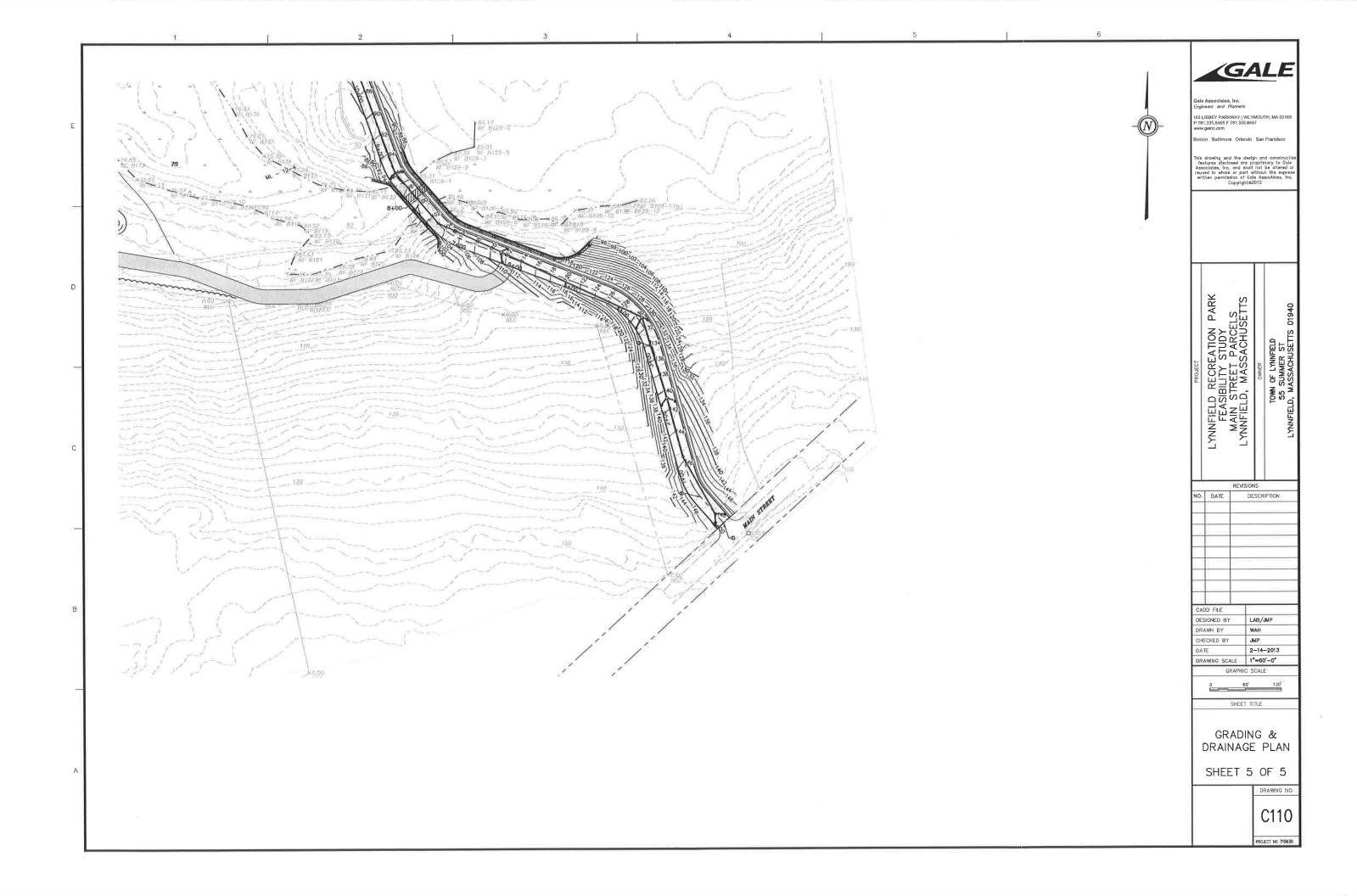


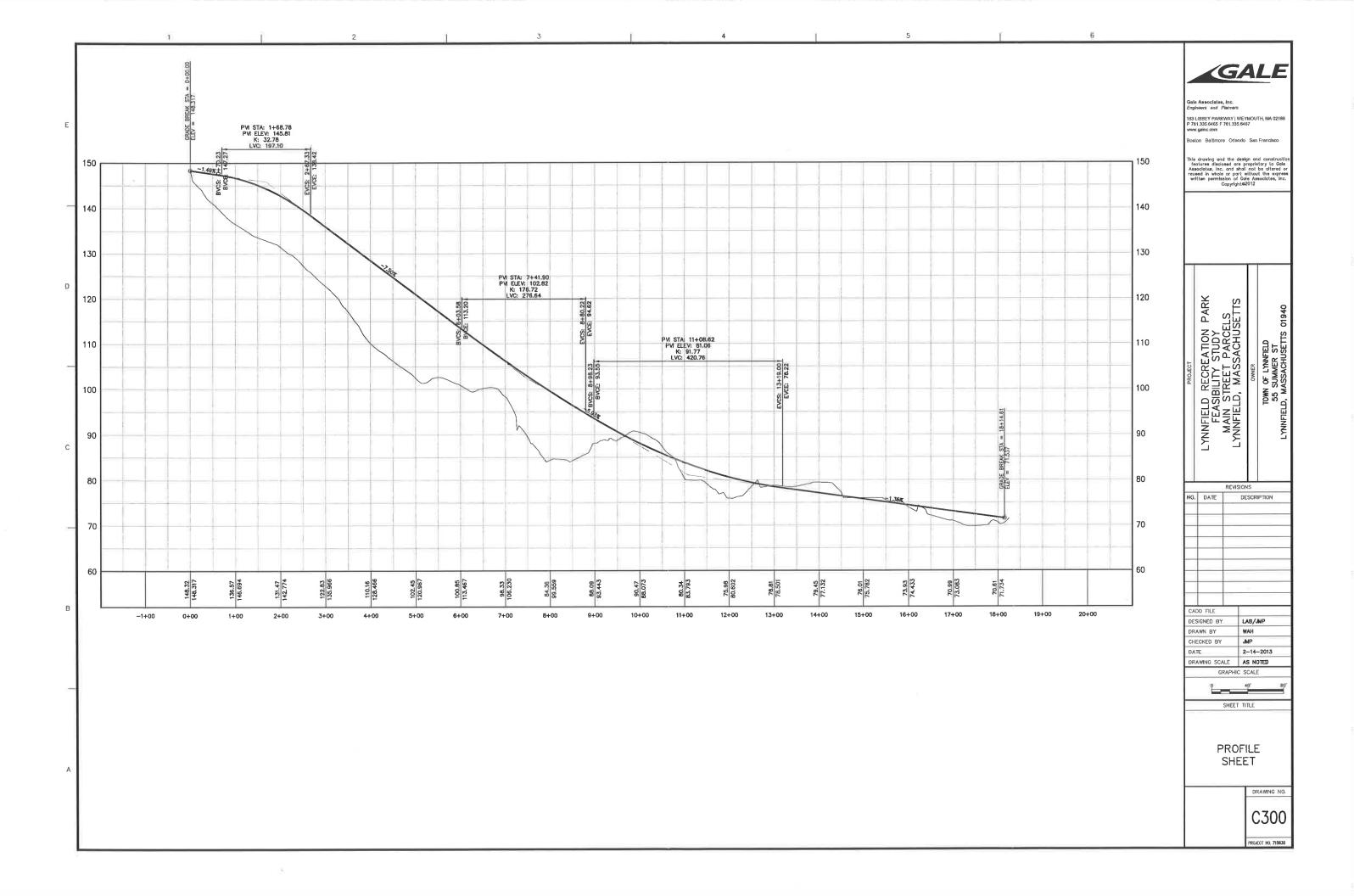












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## Enclosure 8 Traffic Impact Assessment Report

### MEMORANDUM

TO:

Mr. John M. Perry, P.E.

Project Manager Gale Associates, Inc. 163 Libbey Parkway Weymouth, MA 02189

FROM: Mr. Jeffrey S. Dirk, P.E., PTOE, FITE

Principal

Vanasse & Associates, Inc.

10 New England Business Center Drive

Suite 314

Andover, MA 01810-1066 (978) 474-8800 x830 jdirk@rdva.com

DATE:

January 30, 2013

RE:

6305

SUBJECT:

Traffic Impact Assessment Proposed Recreation Facilities Lynnfield, Massachusetts

Vanasse & Associates, Inc. (VAI) has conducted a Traffic Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed recreation park and playing fields to be located along the west side of Main Street and proximate to the Middleton town line in Lynnfield, Massachusetts (hereafter referred to as the "Project"). This study evaluates the following specific areas as they relate to the Project: i) current and projected future traffic volumes; ii) potential impacts on the surrounding roadway network; iii) access and circulation for vehicles, pedestrians and bicyclists along Main Street and at the proposed Project site driveway; and iv) anticipated parking demands.

Based on a review of the findings of this assessment, we have concluded the following with respect to the Project:

- 1. The Project is expected to generate approximately 934 trips on an average weekday (two-way traffic over the operational day of the Project), with approximately 294 vehicle trips expected during both the weekday afternoon/evening and Saturday midday peak hours;
- 2. All movements at the Project site driveway intersection with Main Street were shown to operate with minimal delay (less than 15 seconds) and vehicle queuing (0 to 1 vehicle) during the peak-
- 3. Sufficient parking will be provided within the Project site to accommodate anticipated demands under typical field use;
- 4. No discernible safety deficiencies were noted within the study area based on a review of the motor vehicle crash history along Main Street proximate to the Project site; and
- 5. Lines of sight to and from the Project site driveway intersection with Main Street were found to meet or exceed the required minimum distance for the intersection to function in a safe and efficient manner.



In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner. The following details our assessment of the Project.

### PROJECT DESCRIPTION

As proposed, the Project will entail the construction of a field complex and associated recreational facilities to be located along the west side of Main Street and proximate to the Middleton town line in Lynnfield, Massachusetts. As proposed, the Project will contain three (3) soccer/multi-purpose fields, a tennis/basketball complex, an ice/roller hockey rink, two (2) play areas, two (2) dog parks (one small and one large), a gardening area, walking trails and associated amenities. The Project site encompasses approximately 98.5± acres of land and is generally bounded by the Ipswich River and wooded space to the north; Main Street, residential and agricultural properties, and areas of open and wooded space to the south; Bostik, Inc. and open and wooded space to the east; and areas of open and wooded space to the west. At present, the Project site consists of low-lying wetland areas and open and wooded space. Parking will be provided within the Project site to accommodate 339 vehicles in two separate parking area, with access to be provided by way of a single driveway that will intersect the west side of Main Street approximately 600 feet north of Friendship Lane. Figure 1 depicts the Project site location in relation to the existing roadway network.

#### STUDY METHODOLOGY

This study was prepared in consultation with the Massachusetts Department of Transportation (MassDOT) and the Town of Lynnfield; was performed in general accordance with the Commonwealth of Massachusetts Executive Office of Energy and Environmental Affairs (EEA)/MassDOT Guidelines for Environmental Impact Report/Environmental Impact Statement Traffic Impact Assessments (TIAs), and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area and included an inventory of roadway geometrics; pedestrian and bicycle facilities; public transportation services; observations of traffic flow; and collection of daily and peak period traffic counts.

In the second stage of the study, future traffic conditions were projected and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A five-year time horizon from the current year (2013) was selected for analyses consistent with state guidelines for the preparation of TIAs. The traffic analysis conducted in stage two identifies existing or projected future roadway capacity, traffic safety, and site access issues.

The third stage of the study presents and evaluates measures to address traffic and safety issues, if any, identified in stage two of the study.





Wi

Vanasse & Associates, Inc.

**Site Location Map** 

### **EXISTING CONDITIONS**

A comprehensive field inventory of existing conditions within the study area was conducted in December 2012 and January 2013. The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area for the Project consisted of the Main Street corridor proximate to the Project site.

The following describes existing conditions within the study area.

### Roadway

### **Main Street**

Main Street is a two lane, urban minor arterial roadway under Town jurisdiction that traverses the Town of Lynnfield in a general north-south direction and provides access to Route 62 to the north of the Project site and to Interstate 95 (I-95)/Route 128 to the south. Within the study area, Main Street provides two 13 to 14-foot wide travel lanes separated by a double-yellow centerline with no marked shoulders provided. A sidewalk is currently provided continuously along the south (east) side of Main Street within the study area. Illumination is provided by way of street lights mounted on wood poles. The posted speed limit along Main Street proximate to the Project site is 25 miles per hour (mph) in the northbound direction and 35 mph southbound. Land use along Main Street within the study area consists of the Project site, Bostik Inc., the Sagamore Spring Golf Club, residential properties, and areas of wooded space.

### **Existing Traffic Volumes**

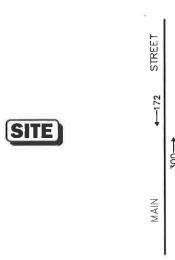
In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts were completed on Main Street in the vicinity of the Project site in December 2012 over a continuous 48-hour period between Friday, December 7<sup>th</sup> and Saturday, December 8<sup>th</sup>, inclusive. It is expected that the peak utilization of the recreational facilities will occur during the weekday afternoon/evening (3:00 to 6:00 PM) and during the Saturday midday period (11:00 AM to 2:00 PM). As such, these time periods were selected for analysis purposes as they would be representative of the peak traffic volume hours for both the Project and the adjacent roadway network.

### **Traffic Volume Adjustments**

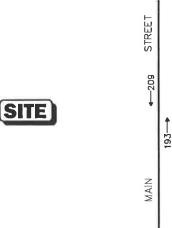
In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, MassDOT weekday seasonal factors for Group 6 roadways (urban arterials, collectors and rural arterials, the MassDOT functional classification for Main Street) were reviewed. Based on a review of this data, it was determined that traffic volumes for the month of December are approximately 3 percent above average-month conditions. As such, the traffic volume data that forms the basis of this assessment was not adjusted downward in order to provide a conservative (above-average) analysis condition. The 2012 Existing traffic volumes are summarized in Table 1, with the weekday evening and Saturday midday peak hour traffic volumes graphically depicted on Figure 2.

<sup>&</sup>lt;sup>1</sup>MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2007 Weekday Seasonal Factors, Group 6 – Urban Arterials, Collectors and Rural Arterials.

### WEEKDAY EVENING PEAK HOUR



### SATURDAY MIDDAY PEAK HOUR







Not To Scale

Vanasse & Associates, Inc. Cisnoportation Engineers & Plannets Figure 2

2012 Existing Peak Hour Traffic Volumes

Table 1 2012 EXISTING TRAFFIC VOLUMES

		Wee	kday Evenin	g Peak-Hour	Sati	ırday M	idday Peak-	-Hour
Location	_ AWTª	VPH <sup>b</sup>	K Factor <sup>c</sup>	Directional Distribution <sup>d</sup>	Saturdaye	VPH	K Factor	Directional Distribution
Main Street, south of Friendship	5,840	472	8.1	63.6% NB	4,670	402	8.6	52.0% SB

<sup>&</sup>lt;sup>a</sup>Average weekday traffic in vehicles per day.

As can be seen in Table 1, Main Street in the vicinity of the Project site was found to accommodate approximately 5,840 vehicles on an average weekday (24-hour, two-way volume), with approximately 472 vehicles per hour (vph) during the weekday evening peak-hour. On a Saturday, Main Street was found to accommodate approximately 4,670 vehicles, with approximately 402 vph during the Saturday midday peak-hour.

A review of the peak-period traffic counts indicates that the weekday evening peak-hour generally occurs between 4:00 and 5:00 PM, with the Saturday midday peak-hour generally occurring between 12:00 and 1:00 PM.

### **Pedestrian and Bicycle Facilities**

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in January 2013. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study intersections, as well as the location of existing and planned future bicycle facilities. A sidewalk is provided continuously along the south (east) side of Main Street within the study area.

At present, no formal existing bicycle facilities were identified within the immediate study area. Further, Main Street does not currently provide sufficient width to accommodate bicycle travel in a shared travelled-way configuration (i.e., motor vehicles and bicyclists sharing the roadway).<sup>2</sup>

### **Public Transportation**

Public transportation services are not directly provided to the Project site or to the Town of Lynnfield; however, the Massachusetts Bay Transportation Authority (MBTA) (Commuter Rail) serves the surrounding Towns of Reading and Wakefield. Reading Station on the Haverhill branch of the MBTA Commuter Rail Line is located at 35 Lincoln Street in Reading to the southwest of the Project site and Wakefield Station on the Haverhill branch is located at 225 North Avenue in Wakefield to the southwest of the Project site (both stations are within a 15 minute driving distance of the Project site), and provide service to North Station in Boston. The MBTA Commuter Rail schedules and fare information are provided in the Appendix.

bVehicles per hour.

<sup>&</sup>lt;sup>c</sup>Percent of daily traffic occurring during the peak hour.

<sup>&</sup>lt;sup>d</sup>Percent traveling in peak direction.

<sup>&</sup>lt;sup>e</sup>Average Saturday traffic in vehicles

NB = northbound; SB = southbound.

<sup>&</sup>lt;sup>2</sup>A 14-foot travel lane is the minimum width required to accommodate both bicyclists and motor vehicles within the same travel lane without requiring that the motor vehicle cross the centerline to pass a bicyclist.

### **Spot Speed Measurements**

Vehicle travel speed measurements were performed on Main Street in the vicinity of the Project site over a continuous 48-hour period in conjunction with the ATR counts. Table 2 summarizes the vehicle travel speed measurements.

Table 2
VEHICLE TRAVEL SPEED MEASUREMENTS

	Main	Street
	Northbound	Southbound
Mean Travel Speed (mph)	34	38
85 <sup>th</sup> Percentile Speed (mph)	39	42
Posted Speed Limit (mph)	25	35

mph = miles per hour,

As can be seen in Table 2, the mean (average) vehicle travel speed along Main Street in the vicinity of the Project site was found to be approximately 36 mph. The 85<sup>th</sup> percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be 40 mph, or approximately 15 mph above the posted speed limit in the northbound direction (25 mph)and 5 mph above the posted speed limit in the southbound direction (35 mph). The 85<sup>th</sup> percentile speed is used as the basis of engineering design and in the evaluation of sight distances, and is often used in establishing posted speed limits.

### Motor Vehicle Crash Data

Motor vehicle crash information for Main Street in the vicinity of the Project site was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent three-year period available (2008 through 2010, inclusive) in order to examine motor vehicle crash trends occurring within the study area. Based on a review of the MassDOT data, no motor vehicle crashes were reported to have occurred at or proximate to this section of Main Street over the three-year review period. As such, the MassDOT data did not indicate a discernible safety deficiency with respect to the segment of Main Street proximate to the Project site.



### **FUTURE CONDITIONS**

Traffic volumes in the study area were projected to the year 2018, which reflects a five-year planning horizon from the current year (2013) consistent with State traffic study guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2018 under No-Build conditions include all existing traffic and general background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2018 No-Build traffic volumes reflect 2018 Build traffic volume conditions with the Project.

### **Specific Development by Others**

The Planning Department of the Towns of Lynnfield and Middleton were contacted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersections. Based on this discussion, there were no developments identified at this time that are expected to result in an increase in traffic within the study area beyond the background traffic growth rate.

### General Background Traffic Growth

Traffic-volume data compiled by MassDOT from permanent count stations and historic traffic counts in the area were reviewed in order to determine general background traffic growth trends. Based on a review of this data, it was determined that traffic volumes within the study area have fluctuated over the past several years, ranging from increases of approximately 0.6 percent to decreases of approximately 1.5 percent. On average, traffic volumes were found to have generally decreased by approximately 0.7 percent per year. Based on this review, a 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

### Roadway Improvement Projects

MassDOT and the Town of Lynnfield were contacted in order to determine if there were any planned roadway improvement projects expected to be completed within the study area. Based on these discussions, no roadway improvements outside of routine maintenance activities were identified to be planned within the study area at this time.

### No-Build Traffic Volumes

The 2018 No-Build condition (without the Project) peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2012 Existing peak-hour traffic volumes. The resulting 2018 No-Build weekday evening and Saturday midday peak-hour traffic volumes are depicted on Figure 3.

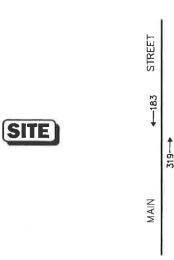
### **Project-Generated Traffic**

In order to develop the traffic characteristics of the Project, trip-generation statistics published by the Institute of Transportation Engineers (ITE)<sup>3</sup> were used. The ITE provides trip-generation information for various types of land uses developed as a result of scientific studies that have been conducted over the past 50 plus years, the most recent update of which was published in 2012. Based on a review of the ITE database and the known elements of the Project, it was determined that the ITE did not have trip-



<sup>&</sup>lt;sup>3</sup>Trip Generation, 9th Edition; Institute of Transportation Engineers; Washington, DC; 2012.

### **WEEKDAY EVENING PEAK HOUR**



### SATURDAY MIDDAY PEAK HOUR



No.

Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure 3

2018 No-Build Peak Hour Traffic Volumes generation information for a similar type facility. Accordingly, the number of proposed parking spaces (339) was used as the basis to develop the anticipated traffic characteristics of the Project.

Traffic volume projections for the weekday evening and Saturday midday peak hours were developed assuming 75 percent utilization of the parking spaces provided and then increased by approximately 10 percent of the available parking supply (approximately additional 40 vehicle trips) to account for drop-off/pick-up activities (i.e., a parent or caregiver that may drop-off/pick-up a child for practice or a game). The directional distribution between entering and exiting traffic was developed based on a review of ITE Land Use Code (LUC) 412, County Park. Table 3 summarizes the anticipated traffic characteristics of the Project, with the detailed trip-generation calculations included in the Appendix.

Table 3
TRIP GENERATION SUMMARY

	Vehicle Trips
	Proposed
Time Period/Direction	Recreation Complex <sup>a</sup>
A 777 1 1 Dellen	
Average Weekday Daily:	167
Entering	467
<u>Exiting</u>	<u>467</u>
Total	934
Weekday Evening Peak Hour:	
Entering	103
Exiting	191
Total	294
	23.
Saturday Midday Peak Hour:	1.00
Entering	168
Exiting	<u>126</u>
Total	294

<sup>&</sup>lt;sup>a</sup>Based on the number of parking spaces to be provided for the Project (339) and using ITE LUC 412, *County Park*, for peak hour directional distribution.

As can be seen in Table 3, the Project was shown to generate approximately 934 vehicle trips (two-way traffic over the operational day of the Project, or 467 vehicles entering and 467 exiting) on an average weekday, with approximately 294 vehicle trips (103 vehicles entering and 191 exiting) expected during the weekday afternoon/evening peak-hour and approximately 294 vehicle trips (168 vehicles entering and 126 exiting) expected during the Saturday midday peak-hour.

### Trip Distribution and Assignment

The directional distribution of generated trips to and from the Project site was determined based on a review of the location of the Project in relation to the population centers within the Town. The general trip distribution pattern for the Project is graphically depicted on Figure 4, with the peak-hour traffic volumes expected to be generated by the Project assigned onto the study area roadway network as shown on Figure 5.



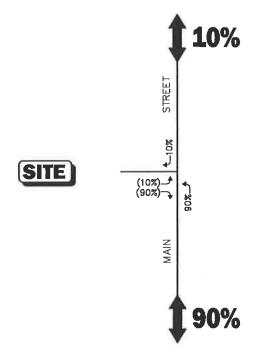




Figure 4

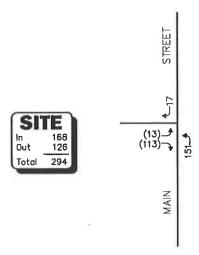
**Trip Distribution Map** 

### WEEKDAY EVENING PEAK HOUR

Legend:

XX Entering (XX) Exiting

### SATURDAY MIDDAY PEAK HOUR





Not To Scale

Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure 5

**Project-Generated Peak Hour Traffic Volumes** 

### **Future Traffic Volumes - Build Condition**

The 2018 Build condition (with the Project) traffic volumes consist of the 2018 No-Build traffic volumes with the additional traffic expected to be generated by the Project added to them. The 2018 Build weekday evening and Saturday midday peak-hour traffic-volumes are graphically depicted on Figure 6.

### TRAFFIC OPERATIONS ANALYSIS

In order to assess the potential impact of the Project on the roadway network, traffic operations and vehicle queue analyses were performed at the intersection of Main Street at the Project site driveway under 2018 Build conditions. The results of the intersection capacity and vehicle queue analyses are summarized in Table 5, with the detailed analysis results presented in the Appendix.

In brief, six levels of service are defined for each type of facility. They are given letter designations ranging from "A" to "F", with a level-of-service (LOS) "A" representing the best operating conditions and a LOS "F" representing congested or constrained operations. A LOS "E" is representative of a transportation facility that is operating at its design capacity with a LOS "D" generally defined as the limit of "acceptable" traffic operations. Since the level-of-service of a traffic facility is a function of the flows placed upon it, such a facility may operate at a wide range of levels of service depending on the time of day, day of week, or period of the year. The Synchro© intersection capacity analysis software, which is based on the analysis methodologies and procedures presented in the 2010 Highway Capacity Manual (HCM)<sup>4</sup>, was used to complete the level-of-service and vehicle queue analyses for the unsignalized study intersection.

Table 5
LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2018	Build	
Unsignalized Intersection/Peak Hour/Movement	Demand <sup>a</sup>	Delay	LOS	Queue 95 <sup>th</sup>
Main Street at the Project Site Driveway				
Weekday Evening:				
Project Site Driveway EB LT/RT	191	11.8	В	1
Main Street NB LT/TH	412	1.8	Α	1
Main Street SB TH/RT	193	0.0	Α	0
Saturday Midday:				
Project Site Driveway EB LT/RT	126	12.2	В	1
Main Street NB LT/TH	356	3.5	A	1
Main Street SB TH/RT	239	0.0	Α	0

<sup>&</sup>lt;sup>a</sup>Demand in vehicles per hour.

<sup>d</sup>Queue length in vehicles.

 $\overrightarrow{EB}$  = eastbound; NB = northbound; SB = southbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

As can be seen in Table 5, under 2018 Build conditions, the critical movements at the Project site driveway intersection with Main Street (all movements from the Project site driveway) were shown to operate at LOS B during both the weekday evening and Saturday midday peak hours. Vehicle queues at

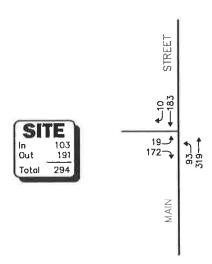


<sup>&</sup>lt;sup>b</sup>Average control delay per vehicle (in seconds).

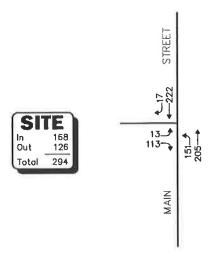
<sup>&</sup>lt;sup>c</sup>Level-of-Service.

<sup>&</sup>lt;sup>4</sup>Highway Capacity Manual, Transportation Research Board; Washington, DC; 2010.

### WEEKDAY EVENING PEAK HOUR



### SATURDAY MIDDAY PEAK HOUR





**Not To Scale** 

(a) Vanasse & Associates, Inc.
Transportation Engineers & Planners

Figure 6

2018 Build Peak Hour Traffic Volumes the intersection were shown to range from 0 to 1 vehicle during the peak periods, with all movements along Main Street shown to operate at LOS A during the peak periods with vehicle queues of approximately 1 vehicle.

Based on the results of this analysis, it has been concluded that the Project site driveway intersection with Main Street will function with limited delay (less than 15 seconds) and vehicle queuing under normal (typical) field utilization and use of the recreational amenities (i.e., non-tournament or special event conditions). This condition is a direct result of the relatively low volume of conflicting traffic on Main Street during the peak hours and that the majority of vehicles exiting the Project will be turning right, a movement that operates with less delay than a left-turn movement. Under special event conditions, it may be necessary to provide a police detail officer to control traffic at the intersection given the concentrated arrivals and departures that typically occur during such events.

### PARKING DEMAND CALCULATIONS

In order to determine the anticipated parking demands of the Project, parking demand data obtained from the ITE<sup>5</sup> for Land Use Code 488, Soccer Complex, a land use with similar amenities to those that will be included as a part of the Project, was used. Table 6 summarizes the average and 85<sup>th</sup> percentile parking demands observed at recreational facilities similar to those of the Project as represented by the ITE data. The 85<sup>th</sup> percentile peak demand is typically used as the design value to determine the adequacy of the parking supply.

Table 6
PARKING DEMAND DATA<sup>a</sup>

Time Period	Average Observed Parking Demand (spaces per field)	85 <sup>th</sup> Percentile Observed Parking Demand (spaces per field)
Weekday Evening	38.30	60.50
Saturday Midday	58.80	65.20

<sup>\*</sup>Based on ITE LUC 488, Soccer Complex.

Applying the ITE parking demand data to the Project (assuming five (5) fields consisting of three (3) multi-purpose fields, an ice/roller hockey rink, and a tennis/basketball court(s)) results in an average peak parking demand for the Project of 192 spaces during the weekday afternoon/evening peak period and 294 spaces during the Saturday midday peak period. The calculated 85<sup>th</sup> percentile peak parking demand for the Project is 302 spaces during the weekday afternoon/evening peak period and 326 spaces during the Saturday midday peak period. Given that the Project site will include parking for a total of 339 vehicles, this analysis has indicated that sufficient parking will be available to accommodate the anticipated parking demands of the Project during the peak parking demand periods, with additional reserve capacity to accommodate parking demands that may be associated with use of the other non-field related amenities that are to be located within the Project site. It should be noted that these calculations are based on simultaneous use of all facilities that are to be located within the Project site.



<sup>&</sup>lt;sup>5</sup>Parking Generation, 4<sup>th</sup> Edition; Institute of Transportation Engineers; Washington, D.C.; 2010.

### SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the Project site driveway intersection with Main Street in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)<sup>6</sup> requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with oncoming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 8 presents the measured SSD and ISD at the subject intersection.

Table 8
SIGHT DISTANCE MEASUREMENTS

Intersection/Sight Distance Measurement	Required Minimum (Feet) <sup>a</sup>	ISD <sup>a</sup>	Measured (Feet)
Main Street at the Project Site Driveway			
Stopping Sight Distance:	2.60		500
Main Street approaching from the north	= = =	**	502
Main Street approaching from the south	360	360 360 430/500 <sup>b</sup>	650+
Intersection Sight Distance:		4= 0 (= 0 ph	0.00
Looking to the north from the Project Site Driveway	360	100100	360
Looking to the south from the Project Site Driveway	360	430/500 <sup>b</sup>	650+

<sup>&</sup>lt;sup>a</sup>Recommended minimum values obtained from A Policy on Geometric Design of Highways and Streets, 6<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011; and based on a 45 mph approach speed on Main Street.

As can be seen in Table 8, the available lines of sight at the Project site driveway intersection with Main Street were found to meet or exceed the recommended minimum sight distance requirement for a 45 mph approach speed along Main Street, consistent with the measured 85<sup>th</sup> percentile vehicle travel speed along this roadway and 10 mph above the posted speed limit in the southbound direction (35 mph) and 20 mph above the posted speed limit in the northbound direction (25 mph).

bValues shown are the intersection sight distance for a vehicle turning right/left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed.

<sup>&</sup>lt;sup>6</sup>A Policy on Geometric Design of Highway and Streets, 6<sup>th</sup> Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011.

### CONCLUSIONS AND RECOMMENDATIONS

### Conclusions

VAI has conducted a TIA in order to determine the potential impacts on the transportation infrastructure associated with the proposed recreation park and playing fields to be located along the west side of Main Street and proximate to the Middleton town line in Lynnfield, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) current and projected future traffic volumes; ii) potential impacts on the surrounding roadway network; iii) access and circulation for vehicles, pedestrians and bicyclists along Main Street and at the proposed Project site driveway; and iv) anticipated parking demands.

Based on a review of the findings of this assessment, we have concluded the following with respect to the Project:

- 1. The Project is expected to generate approximately 934 trips on an average weekday (two-way traffic over the operational day of the Project), with approximately 294 vehicle trips expected during both the weekday afternoon/evening and Saturday midday peak hours;
- 2. All movements at the Project site driveway intersection with Main Street were shown to operate with minimal delay (less than 15 seconds) and vehicle queuing (0 to 1 vehicle) during the peakhours;
- 3. Sufficient parking will be provided within the Project site to accommodate anticipated demands under typical field use;
- 4. No discernible safety deficiencies were noted within the study area based on a review of the motor vehicle crash history along Main Street proximate to the Project site; and
- 5. Lines of sight to and from the Project site driveway intersection with Main Street were found to meet or exceed the required minimum distance for the intersection to function in a safe and efficient manner.

In consideration of the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner.

### Recommendations

The following recommendations are offered for consideration with respect to the design and operation of the Project with specific regard to the proposed driveway and traffic and parking management.

- > The Project site driveway should be a minimum of 24 feet in width (28 feet if bicycle accommodations are to be afforded) and accommodate two-way travel, with vehicles exiting the Project site placed under STOP-sign control.
- A leveling area not exceeding 2 percent in grade should be provided on the driveway for a minimum distance of 100 feet (approximately four (4) vehicles) approaching Main Street.
- > Consideration should be given to providing a secondary means of access to the field complex for emergency vehicles given the length of the proposed driveway and remote nature of the fields with respect to Main Street. This condition should be reviewed in consultation with the



Lynnfield Police Department and Fire Department with respect to their requirements for response to the Project.

- ▶ It is suggested that a sidewalk be provided along one or both sides of the driveway, with consideration to locating a crosswalk for crossing Main Street at the Project site driveway intersection. If a crosswalk is provided, the crossing should include wheelchair ramps and the installation of pedestrian crossing warning signs at and in advance of the crossing (175 feet north and south of the crosswalk location).
- Where provided, pedestrian crossings within the Project site should also include a marked crosswalk, wheelchair ramps and the installation of pedestrian crossing warning signs at the crossing.
- > Bicycle racks should be provided at appropriate locations within the Project site.
- > Signs and landscaping adjacent to the Project site driveway intersection with Main Street and within the Project site should be designed and maintained so as not to restrict lines of sight.
- Within the Project site, roadways should be designed with appropriate geometry (horizontal and vertical) to accommodate a safe travel speed of 25 mph.
- > Centerline pavement markings, where provided, shall consist of a double-yellow line in accordance with the centerline pavement marking standards of the *Manual on Uniform Traffic Control Devices* (MUTCD).<sup>7</sup>
- > All signs and other pavement markings to be installed within the Project site shall conform to the applicable standards of the MUTCD.
- Police Department for special event conditions such as tournaments, clinics and other such use of the amenities that are to be located within the Project site where the expected attendance level may exceed the available parking supply (339 spaces) or as defined by the Police Department. The management plan should include measures to encourage car/vanpooling, buses and other means of transportation that are designed to reduce the overall traffic and parking demands of the Project. Further, the scheduling of games and practices should also be coordinated in order to stagger the arrival and departure of traffic. For major events, police detail officers may be required in order to manage the flow of traffic and the safety of pedestrians both within the Project site and along Main Street. The use of police details should be at the discretion of the Police Chief and in consultation with the appropriate Town officials.

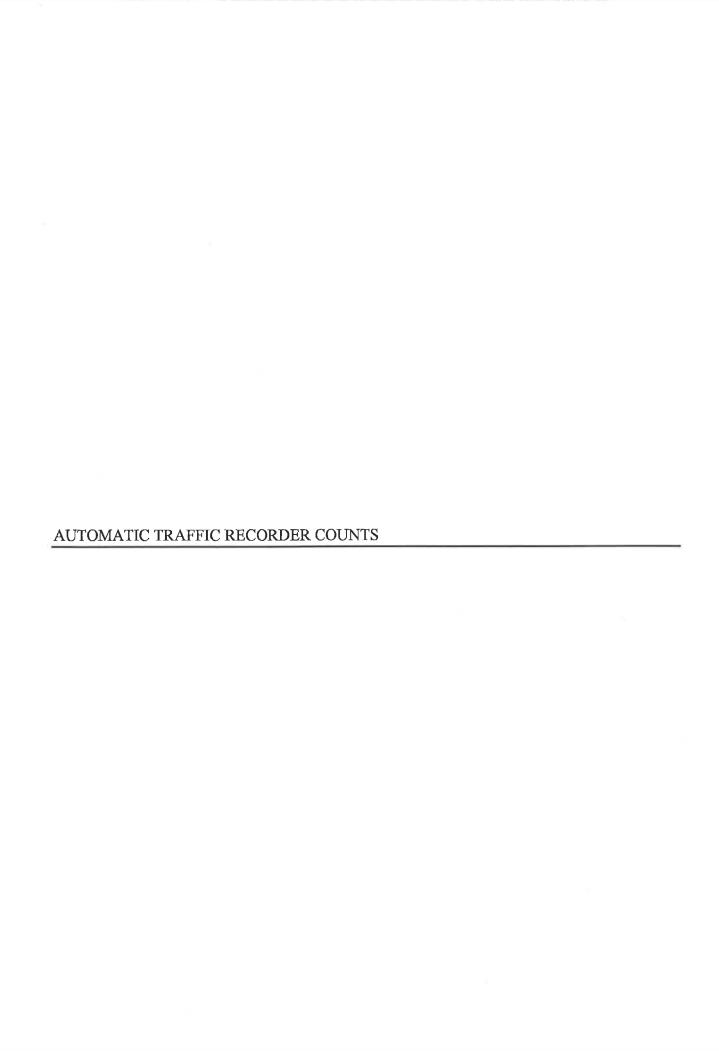
With implementation of the above recommendations, safe and efficient access will be provided to the Project site and the Project can be constructed with minimal impact on the roadway system.

<sup>&</sup>lt;sup>7</sup>Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, DC; 2009.



### **APPENDIX**

AUTOMATIC TRAFFIC RECORDER COUNTS SEASONAL ADJUSTMENT DATA PUBLIC TRANSPORTATION SCHEDULES VEHICLE TRAVEL SPEED DATA GENERAL BACKGROUND TRAFFIC GROWTH TRIP-GENERATION CALCULATIONS CAPACITY ANALYSIS WORKSHEETS



Location: Main Street Location: South of Friendship Lane City/State: Lynnfield, MA

Start	07-Dec-12		NB	Hour	Totals		SB	Hour	Totals		ed Totals
Time	Fri	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoo
12:00		8	35			2	43				
12:15		9	38			5	40				
12:30		9	34			1	48				
12:45		1	50	20	157	1	41	9	172	29	32
01:00		0	50			0	37				
01:15		5	45			0	34		1		
01:30		Õ	37			0	50				
01:45		1	43	6	175	1	50	1	171	7	34
02:00		i	40	·		2	34				
02:15		Ó	69			1	35				
02:30		0	50			3	43				
02:45		2	67	3	226	3 1	42	7	154	10	38
03:00		1	68	J	220	Ó	39	·			
03:15		i	78			1	58				
03:30		Ö	65			'n	47		-		
03:45		2	70	4	281	2	44	3	188	7	46
04:00		1	73	7	201	2	43	Ů	.00	•	
04:00		Ó	77		-	3	39				
04.13		1	73			0 2 2 3 6	48		1		
		1	77	3	300	4	42	15	172	18	47
04:45		2	67	3	300	11	42	10	,,,_	,,,	
05:00		2	76		1	12	48				
05:15		2 6			- 1	26	55		1		
05:30		0	68	16	281	39	43	88	188	104	46
05:45		6	70	10	201	49	44	00	100	104	40
06:00		8	73		- 1	49	38				
06:15		11	72		- 1	68	30		1		
06:30		8	59	10	004	60	28	264	142	310	40
06:45		19	57	46	261	87	32 33	204	142	310	40
07:00		23	42		- 1	81	33				
07:15		17	29		1	70	32		l l		
07:30		31	40			83	19	0.00	440	000	٥٥
07:45		3.0	29	101	140	64	34	298	118	399	25
08:00		25	27			53	16		1		
08:15		30	24			74	19		- 1		
08:30		43	25			72	12				40
08:45		45	18	143	94	61	25	260	72	403	16
09:00		34	17			54	17		- 1		
09:15		33	17			40 33	14		1		
09:30		34	20			33	11				
09:45		36	22	137	76	33	13	160	55	297	13
10:00		24	17			30	14		- 1		
10:15		32	19		- 1	43	22		- 1		
10:30		31	14			40	8				
10:45		28	22	11:5	72	48	5	161	49	276	12
11:00		46	12			51	13		1		
11:15		35	16			47	13		1		
11:30		37	9			46	9				
11:45		32	11	150	48	54	4	198	39	348	8
Total		744	2111		11034	1464	1520			2208	363
Percent		26.1%	73.9%			49.1%	50.9%			37.8%	62.29

Location: Main Street

Location: South of Friendship Lane

City/State: Lynnfield, MA

Start	08-Dec-12	N	IB	Hour	Totals		B	Hour	Totals	Combine	ed Totals
Time	Sat	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon		Afternoon	Morning	Afternoor
12:00		18	47				50				
12:15		5	47		1	5	37				
12:30		5 7	47			5 5 6 1	54				
12:45		7	52	37	193	1	68	17	209	54	40
12.40		,	32	31	193	1	49	- 17	203	34	70
01:00		0	47			-1	49				
01:15		3	43			2	44				
01:30		3	45			0	51	_			
01:45		1	55	7	190	4	52	7	196	14	38
02:00		3	36			1	35		1		
02:15		2	37		- 1	1	54		- 1		
02:30		3	47		1	0	57				
02:45		1	47	9	167	0	48	2	194	11	36
03:00		i	43	•		1	42	_			
03:15		i	46			Ö	43				
03:15			49		- l	1	46		- 1		
		0	49	-	100		54	2	185	4	3.6
03:45		0	42	2	180	0 0 1		Z	100	4	3.0
04:00		1	39			U	50		- 1		
04:15		1	39				36		- 1		
04:30		0	38			1	42		- 1		
04:45		0	39	2	155	2 2 4	45	4	173	6	32
05:00		3	43			2	39		Í		
05:15		2	53			4	34		- 1		
05:30		2 4	43			11	34		1		
05:45		3	34	12	173	8	45	25	152	37	32
06:00		1	34 37	-	,,,,	7	46			•	-
00.00			47			10	39		- 1		
06:15		3	4/			10	39		- 1		
06:30		7	30	4.5	450	12	30	4.5	A 4-7	20:	
06:45		4	36	15	150	16	32	45	147	60	29
07:00		9	16			10	28		- 1		
07:15		7	28			15	33		- 1		
07:30		8	24 21			12	28				
07:45		16	21	40	89	16	23.	53	112	93	20
08:00		17	21			25	23				
08:15		20	23		1	30	16				
08:30		23	23 26		1	34	20				
08:45		23	17	83	.87	45	20	134	79	217	16
00:45		23	1/	03	.07	33	19	194	13	217	1.0
09:00		29	19			33	19		1		
09:15		28	17			31	18				
09:30		24	20			35	19				
09:45		41	19	122	75	41	13	140	69	262	14
10:00		25	24		1	34	15				
10:15		35	24			45	15				
10:30		33	15			32	15		1		
10:45		44	15	137	78	47	9	158	54	295	13
11:00		52	13			57	18		- /		
11:15		46	15		- 1	50	16				
44.00					- 1	55	13				
11:30		44	16	400	50			247	50	200	40
11:45		40	8	182	52	55	9	217	56	399	10
Total		648	1589			804	1626			1452	32
Percent	_	29.0%	71.0%			33.1%	66.9%			31.1%	68.9
Grand		1392	3700			2268	3146			3660	684
Total											
Percent		27.3%	72.7%			41.9%	58.1%			34.8%	65.2

ADT

ADT 5,253

AADT 5,253

Accurate Counts 978-664-2565

Location: Main Street Location: South of Priendship Lane City/State: Lynnfield, MA

	03-Dec-12	12	Tue		Wed		로		F		Sat		Sun		Week Average	/erage
	SB B	SB	NB	S	88	SB	NB NB	SB	9	SB	S N	SB	g	SB	8	SB
M	•	*	•)		•	1 2	•	•	20	G	37	17		٠	28	13
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03:00	•	*	٠	•	: <b>#</b> /	*	٠	•	4	ო	7	7	*	•	ന	2
04:00	•	*	٠	*	*:	*	•	٠	ო	70	2	4	46	Þ	7	10
02:00	•	+		*	٠	*	•		16	80	12	25	<b>#</b> I		14	28
00:00	•	*	٠	*	*	*	*	*	46	264	15	45	*		30	154
07:00	•	*		•	*	*	ě	٠	101	298	40	23	•	•	202	176
08:00	4	*	٠	٠	*	*	•	٠	143	260	83	134	•	•	113	197
00:60	9	+		×	×	*	٠	٠	137	160	122	140	٠	•	130	150
10:00	٠	-ju		٠	*	*	٠	,	115	161	137	158	٠	*	126	160
11:00	•	*	٠	٠	*	*	•	٠	150	198	182	217	•	•	166	208
MG 00-	•	*	٠	٠	¥	*	٠	*	157	172	193	209	٠	•	175	190
01:00	•	+	٠	H	•	*		*	175	171	190	196	•	•	182	184
05:00	*	*			÷	•	٠	٠	226	154	167	194	٠	•	196	174
03:00	•	-ju		٠	*	*	*	٠	281	188	180	185	*	•	230	186
04:00		*	٠	•	*	٠	<b></b>	٠	300	172	155	173	•	*	228	172
05:00		*	•	Y	*	•	•	•	281	188	173	152	•	4	227	170
06:00	•	je.	•	:W	*	*	٠	•	261	142	150	147	٠	*	206	144
07:00	,	*	٠			*		•	140	118	89	112	•	•	114	115
08:00	•	*	٠	*	*	*		٠	8	72	87	62	•	•	90	76
00:60	•	*	*	*	•	•	•	٠	92	55	75	69		•	9/	62
10:00	•	+			*			*	72	49	78	22	•	•	75	22
11:00		4		×		*		•	48	39	52	99		•	20	48
Lane	0	0	0	0	0	0	0	0	2855	2984	2237	2430	0	0	2543	
Day	0		0		0		0		5839		4667		0		5250	
AM Peak	٠	٠			•)	¥6	•	٠	11:00	07:00	11:00	11:00	30	•	11:00	11:00
Vol.		3		1	,	1		1	150	298	182	217	*		166	208
PM Peak	٠	٠		(10)	,		30	¥	16:00	15:00	12:00	12:00		•	15:00	12:00
Vol.	,	i	į				1	¥.	300	188	193	209	: <b>(</b> )	•	230	190
1																
Total	0		0	_	0	_	0		£.	5839	4	4667		0	ις.	5250
ADT	FDA	ADT 5,253	AADT	AADT 5,253												



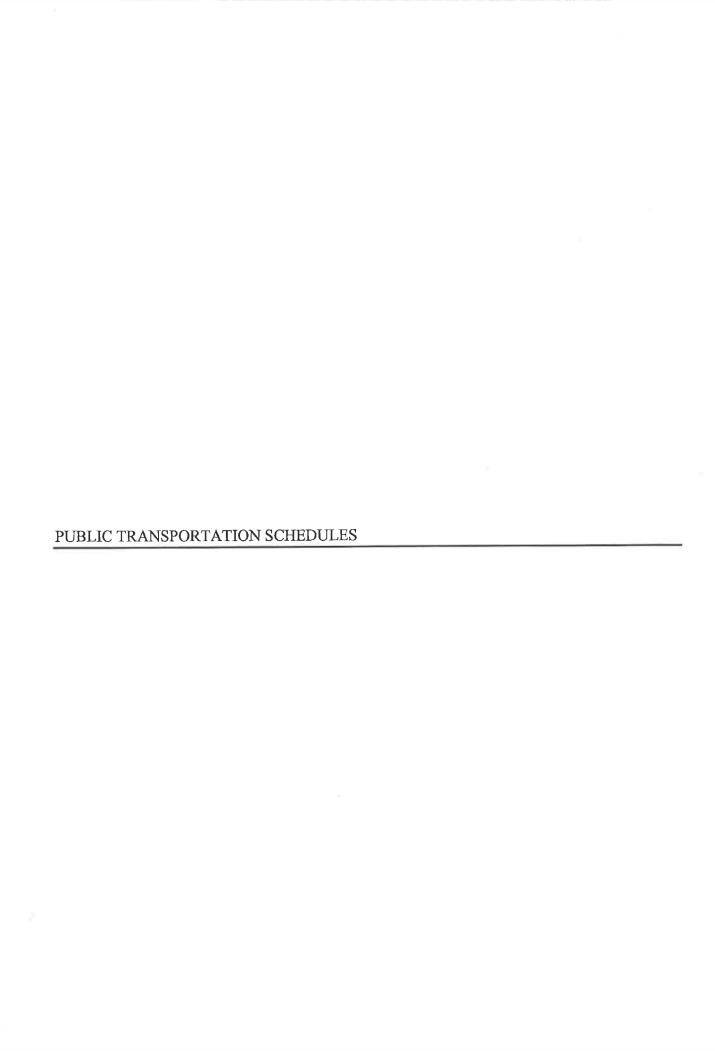
# MASSACHUSETTS HIGHWAY DEPARTMENT - STATEWIDE TRAFFIC DATA COLLECTION

# 2007 WEEKDAY SEASONAL FACTORS \*

<sup>\*</sup> Note: These are weekday factors. The average of the factors for the year will not equal 1, as weekend data are not considered.

FACTOR GROUP	JAN	FEB	MAR	APR	MAY	NOC	JU.	AUG	SEP	500	>ON	DEC
GROUP 1 - WEST INTERSTATE	0.95	0.91	0.85	0.85	0.87	0.86	0.91	96.0	0.90	0.88	0.30	0.91
COCCUSATION MAIOD COLLECTOR (P.5)	1 + 1	1 07	1.07	0.98	0.92	0.88	0.88	0.86	0.89	0.93	1.01	1.04
GROOF 2 - NORTH WAVON COLLECTOR (1953)	1.26	1 20	18	40	0.96	0.86	0.78	0.79	0.93	0.99	1.07	1.12
GROUP 3A - RECREATIONAL (1-4) 386 below	122	2 2	1.20	1.04	0.96	0.88	0.73	0.74	0.99	1.02	1.12	1.17
CHOOL OF THE NITE OF A SHOW	20 20	1 03	103	0.95	0.93	0.87	0.86	0.83	0.89	0.93	0.93	0.96
GROUP 4-1-450 IN ERO IN E		0 0	79 U	46.0	0.95	0.91	0.92	0.92	0.94	0.94	0.98	0.99
GROUT 0 - EACH INTERCIALE	103	99.0	0.97	0.92	0.91	0.90	0.92	0.91	0.92	0.93	76.0	0.97
GROUP 6 - URBAN ARTERIALS, COLLECTORS & RUPAL ARTERIALS (N.Z. 173)	0.84	1 15	1.17	1.08	1.10	1.02	1.01	0.96	1.06	1.06	1.11	1.15
GEOUR 1-1-04 PROXIMITY (STA 6590)	0.95	1.01	0.96	0.92	0.89	0.88	0.91	0.86	0.91	0.93	0.95	0.92
GROUP 9 - 1-195 PROXIMITY (STA 7)	1.10	1.03	1.00	0.94	0.91	0.87	0.84	0.82	0.88	0.93	1.03	0.99

RECREATIONAL: (ALL YEARS)	2007 AXLE CORRECTION FACTORS	ON FACTORS	ROUND OFF
**GROUP 3A: 1. CAPE COD (ALL TOWNS) 2.PLYMOUTH(SOUTH OF RTE.3A)	ROAD INVENTORY FUNCTIONAL CLASSIFICATION RURAL	AXLE CORRECTION FACTOR	0 - 99910 > 1,00010
7014, 7079,7080,7091,7092,7093,7094,7095,7096,7097,7108,7178 3.MARTHA'S VINEYARD 4.NANTUCKET	1 2 3 0,5,6 URBAN 1	0.00 0.93 0.98 0.98 0.96	
***GROUP 3B: 5.PERMANENTS 2 & 189 1065,1067,1083,1084,1085,1086,1087,1088,1089,1090,1091,1082, 1083,1094,1085,1096,1097,1098,1099,1100,1101,1102,1103,1104, 1105,1105,1107,1108,1114,1115,2196,2197,2198	2,3 0.97 5 0.99 · 0,6 0.99 1-84 0.83	0.99 0.99 0.99 0.83	
	STOLIS OF TACION 12 STATIONS	387.387	





# Monday through Friday

bound to North Station

			Haverhill	Bradford	Lawrence	Andover	Bailardvale	3 North Wilmington	Wilmington	Anderson/Woburn	Reading	Waterfield	Greenwood	Melrose Highlands	Metrose/Cedar Park	Wyomina Hill	A Maiden Center	
96	202	A.M.	SG	5 08	5.19	5.24	f\$30	1537	1	nco	244	5 50	1553	5 55	T	t	1.602	6.14
	20	A.M.	5.38	5 41	2.2	5.57	603	611	1	1	619	6.25		6 30	633	⊢	H	6.50
	907	A.M.	6.10	613	624	6 29	6.35	643	1	1	651	6.57	7 00	7 03	7.05	H	F	
	22	A.M.	9 4 9	6 49	2.00	307	712	1	722	1	1	N.	Lowell	Time	1	1	1	7.48
	20	A.M.	1	1	1	1	1	1	1	ı	730	7.36	7.35	742	7.44	7.46	1.750	8 00
	992	A.M.	1	1	1	1	1	1	1	1	00 8	8 06	809	8 12	77.0	1 16	<u>=</u>	8 30
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	292	A.M.	1	1	1	1	1	1	1	1	6.30	8 36	839	8 42	17.0	8.46	1.830	9 00
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8	230	P.M.	J	1	1	ļ	1	1	ţ	1	432	4.38	ı	1441	*	1	L447	4 58
8	232	P.M.	4 30	14.33	4 44	f449	14.55	1	ı	f5 11	1	N:S	Lowell	Line	1	ı	1	5 32
9	282	P.M.	1	ŧ	ı	-	1	ŧ	1	1	5 35	5.40	5.43	\$45	5 47	5.49	1553	6.04
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8	338	P.W.	8 25	f 8 28	1839	1844	8 50	153	1	I	9 OM	80 6	1912	1914	f 9 16	1918	1922	9.33
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N. N.	NORTH STATION	6.45	7.15	7.35	7.50	8 04	9.25	10.30	12.20	1.10	220	3 00	350	4.30	449	5.15	5.35	525	6.20	6 55	7.20	B 40	9.35	10 35	12 10
2	Maiden Center	35.9	17.25	7.45	1	£8 14	f 9.35	10.40	12.30	1 20	230	3.10	4 01	441	4 59	5.25	5.45	50.9	630	1	7 30	8 50	9 45	10.45	12 20
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# Shaded area indicates peak hour trains.

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Train No. Saturdays Train No. Sundays NORTH STATION Malden Center

922 1224 2224

2121 2122

1208 2208

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Train No. Saturdays Train No. Sundays

Zones

Saturday and Sunday

Wyoming Hill Melrose/Cedar Park Melrose Highlands

10 24 10 29 f 10 34

10 23 10 32 10 37 f 10 42

(6) 7 Haverhill
(5) 7 Bradford
(5) 6 Lawrence
(5) 5 Andover
(5) 4 Ballardvale
(7) 1 Morth Wilmin

f 10 40

f 7 48

757 1742

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f 10 48

North Wilmington

Ballardvale

11.07

Malden Center NORTH STATION

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Wakefield

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Makerfield
 Makerfield
 Greenwood
 Melrose Highlands
 Melrose Highlands
 Melrose Cadar Park
 Myoming Hill



# massDOT MBCR

# Notes: This schedule is effective July 1, 2012 and replaces the schedule of January 11, 2010.

Times shown in this schedule are train departure times; customers are asked to arrive at the station platform for a prompt departure.

### Holiday Service:

Trains will operate on a Saturday or Sunday service on the holidays indicated:

Saturday Service:
President's Day and 4<sup>th</sup> of July (train no. 1225 may be held for 30 minutes after the conclusion of the 4<sup>th</sup> of July fireworks)

### Sunday Service:

New Year's Day, Memorial Day, Labor Day, Thanksgiving Day and Christmas Day.

### All other holidays:

Updates' at www.mbta.com or call Customer Service at 617-222-3200 for any extra service that may be Regular service is provided on all other holidays, Consult 'Service provided.

# Times in red indicate an f stop-

off at these designated stations.
Passengers who wish to board at these designated stations must be on the platform in full view of the Passengers must notify the conductor that they wish to get

# Times in blue indicate an L stop

This is a regular stop to discharge or pick up passengers; however the train may leave ahead of schedule.

# VIA LOW- operates via the Lowell line between Wilmington and North Station. See the Lowell line schedule

ه Bicycles are allowed on trains with the bicycle symbol shown above the train number.

for all stops.

VEHICLE TRAVEL SPEED DATA

Location : South of Friendship Lane City/State: Lynnfield, MA Northbound Location: Main Street

 $\frac{4}{2}$ 11:00 16:00 777 13:00 08:00 08:00 15:00 33.0% 64 11:00 11:00 14.0% 08:00 15:00 Start Time 12/07/12 01:00 03:00 03:00 04:00 04:00 05:00 05:00 05:00 05:00 05:00 11:0

Location: Main Street
Location: South of Friendship Lane

City/State: Lynnfield, MA

11:00 0.0% 0.0% 0.0% 999 0.0% 11:00 12:00 2061 40.5% 28 MPH 33 MPH 39 MPH 42 MPH 11:00 18:00 15th Percentile : 50th Percentile : 85th Percentile : 95th Percentile : 18:00 100 2.0% 07:00 31 11:00 13:00 Time 12/08/12 01:00 03:00 03:00 05:00 05:00 05:00 05:00 05:00 05:00 05:00 11:0 AM Peak Start

30-39 MPH 3368 66.1% 2093 41.1% 34 MPH

Number of Vehicles > 35 MPH.
Percent of Vehicles > 35 MPH
Mean Speed(Average)

MPH Pace Speed: Number in Pace: Percent in Pace:

Stats

Location: Main Street Location: South of Friendship Lane City/State: Lynnffeld, MA Southbound

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03:00	0	0	·	0	0	0				0	0	0	0	0	က	44	45
04:00	0	0	0	0	-	7				-	0	0	0	0	15	46	56
02:00	0	0	0	0	13					0	0	0	0	0	88	44	47
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17:00	9	-	_	7	52					0	0	0	0	0	188	40	42
18:00	ဖ	•	_	00	46					0	0	0	0	0	142	39	45
19:00	4	0	-	7	42					0	0	0	0	0	118	4	43
20:00	0	7	7	4	30					0	0	0	0	0	72	40	43
21:00	-	0	0	_	16					٥	0	0	0	0	22	4	43
22:00	-	0	0	7	9					0	0	0	0	0	49	4	43
23:00	0	0	2	မ	12			- 1		0	0	0	0	0	33	39	4
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Vol.	9	2	2	Ø	52	86	75	18	2	~					188		

Location: Main Street
Location: South of Friendship Lane
City/State: Lynnfield, MA
Southbound

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	15th P. 95th P	15th Percentile: 50th Percentile: 85th Percentile: 95th Percentile:	32 MPH 37 MPH 42 MPH 45 MPH	<b>.</b>											
			)												
Stats 10 MPH Pace Speed Number in Pace Percent in Pace Number of Vehicles > 35 MPH Percent of Vehicles > 35 MPH	MPH Pace Speed Number in Pace Percent in Pace /ehicles > 35 MPH	PH Pace Speed:  Number in Pace:  Percent in Pace: icles > 35 MPH:	34-43 MPH 3646 67.3% 4074 75.2%	I 0 % 4 ~											
Mea	Mean Speed(Average)	(verage)	38 MPH	2 7"											

Location: Main Street
Location: South of Friendship Lane
City/State: Lynnfield, MA
Northbound, Southbound

	95th	Percent	43	43	47	45	54	47	46	46	46	46	46	45	46	45	46	44	43	41	4	4	41	42	42	41						
	85th	Percent	40	39	43	43	45	4	42	43	43	42	42	42	43	42	42	4	40	38	38	38	38	39	39	38						
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	99	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	%0.0				
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	28	30	4	0	0	0	7	_	13	19	∞	12	19	25	4	19	21	20	20	51	7.7	35	38	60	18	3	477	8.2%	11:00	25	18:00	77
	24	25																										1.3%	_	က	17:00	13
	16	20	0	0	0	0	0	0	0	0	7	-	-	0	0	-	0	4	0	-	7	2	4	0	0	0	18	0.3%	08:00	2	15:00	4
punagunos	ς-	15	0	0	0	0	0	0	0	9	00	ო	4	ဖ	7	~	7	18	12	17	17	ဖ	· <del>-</del>	-	•	0	115	2.0%	08:00	80	15:00	18
	Start	Time	12/07/12	01:00	02:00	03:00	04:00	02:00	00:90	07:00	08:00	00:60	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	Percent	AM	Vol.	PM	Vol.

Location: Main Street
Location: South of Friendship Lane
City/State: Lynnfield, MA
Northbound: Southbound

Start 1 16 2 1 2 2 2 2 2 3 0 3 4 1 46 5 1 56	Northbound, Southbound	uthbound																
15   20   25   30   35   40   45   50   55   60   65   70   75   99   104a   Percent	Start	τ-	16	21	56	31	36	41	46	57	56	61	99	71	76		85th	95th
1	Time	15	20	25	30	35	40	45	20	22	9	65	<u>7</u> 0	75	666		Percent	Percent
1	2/08/12	0	0	7	12	23	12	4		0	٥	0	0	0	0		38	42
10   10   10   10   10   10   10   10	01:00	0	0	_	0	∞	4	_		0	0	0	0	0	0		38	41
1	05:00	0	0	7	က	-	4	-		0	0	0	0	0	0		39	42
1	03:00	0	0	0	<del>-</del>	ო	0	0		0	0	0	0	0	0		34	35
0         1         2         10         15         8         1         0	04:00	0	0	0	0	2	ო	0				0	0	0	0		49	23
2         0         1         6         13         24         40         21         1         0 <td>02:00</td> <td>0</td> <td>0</td> <td>-</td> <td>2</td> <td>10</td> <td>15</td> <td>∞</td> <td></td> <td></td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td>42</td> <td>44</td>	02:00	0	0	-	2	10	15	∞				0	0	0	0		42	44
1	00:90	2	0	-	ဖ	13	25	12				0	0	0	0		4	44
1	02:00	0	0	2	m	24	40	21				0	0	0	0		42	45
1	08:00	n	0	0	5	51	94	44				0	0	0	0		42	45
4         2         2         1         88         128         48         11         0 <td>00:60</td> <td>-</td> <td>0</td> <td>0</td> <td>#</td> <td>67</td> <td>129</td> <td>43</td> <td></td> <td></td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td>41</td> <td>45</td>	00:60	-	0	0	#	67	129	43				0	0	0	0		41	45
9         0         16         103         167         18         12         5         0         0         0         0         0         0         0         0         0         0         0         0         0         0         402         42         <	10:00	4	7	2	12	88	128	48				0	0	0	0		41	45
12   0   0   18   18   18   17   17   19   0   0   0   0   0   0   0   0   0	11:00	6	0	0	<del>1</del> 0	103	167	88				0	0	0	0		42	45
12 0 1 1 25 94 173 72 9 0 0 0 0 0 0 386 41 11 1 0 2 12 12 107 168 58 5 7 1 0 0 0 0 0 0 0 386 41 11 1 0 2 12 12 107 168 58 5 0 0 0 0 0 0 0 0 386 41 12 0 3 29 124 126 32 5 0 0 0 0 0 0 0 0 0 386 39 13 0 86 134 98 20 0 0 0 0 0 0 0 0 297 38 13 0 86 64 13 14 12 10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 PM	00	0	0	18	88	189	8				0	0	0	0		42	45
11	13:00	7	0	· <del>-</del>	52	8	173	72				0	0	0	0		4	44
Fig. 1   3   23   101   168   58   5   0   0   0   0   0   0   0   0   0	14:00	7	0	7	12	107	159	62				0	0	0	0		41	44
9 0 0 3 2 29 124 126 32 5 0 0 0 0 0 0 0 328 39 39 39 39 3	15:00	9	-	ო	23	101	168	58				0	0	0	0		4	44
1         0         3         63         132         86         36         3         1         0 <td>16:00</td> <td>σ</td> <td>0</td> <td>ო</td> <td>29</td> <td>124</td> <td>126</td> <td>32</td> <td></td> <td></td> <td></td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td></td> <td>39</td> <td>43</td>	16:00	σ	0	ო	29	124	126	32				0	0	0	0		39	43
3         0         6         36         134         98         20         0         0         0         0         0         297         38           1         0         5         30         86         64         13         1         0         0         0         0         0         0         0         0         0         144         38           2         2         3         21         67         45         4         0         0         0         0         0         144         38           2         1         6         15         54         41         12         0         0         0         0         0         144         38           76         6         43         392         1496         1864         681         101         8         0         0         0         0         0         146         <	17:00	-	0	ო	63	132	86	36				0	0	0	0		39	43
2 0 5 30 86 64 13 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	18:00	ო	0	9	36	134	86	20				0	0	0	0		38	41
1 0 0 0 31 67 59 7 1 0 0 0 0 0 0 0 0 166 38  2 1 2 3 21 67 59 7 1 0 0 0 0 0 0 0 0 144 38  2 1 4 41 12 1 0 0 0 0 0 0 0 144 38  2 1 5 44 41 12 1 0 0 0 0 0 0 0 0 144 38  1 6 43 392 1496 1864 681 101 8 0 0 0 0 0 0 0 0 132 39  1 6 43 392 1496 1864 681 101 8 0 0 0 0 0 0 0 0 0 0 108  1 7 6 6 43 392 1496 1864 681 101 8 0 0 0 0 0 0 0 0 0 0 0 0 108  1 7 6 6 43 392 1496 1864 681 101 8 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	19:00	7	0	S	30	86	64	13				0	0	0	0		38	4
2         2         3         21         67         45         4         0         0         0         0         0         0         144         38           2         1         6         15         54         41         12         1         0         0         0         0         0         0         132         39           76         6         43         392         1496         1864         681         101         8         0         0         0         0         0         0         0         108         40         108         0	20:00	~	0	0	31	29	59	7				0	0	0	0		38	4
2 1 6 15 54 41 12 1 0 0 0 0 0 0 132 39  76 6 43 392 1496 1864 861 101 8 0 0 0 0 0 0 108 40  1.6% 0.1% 0.9% 8.4% 32.1% 39.9% 14.6% 2.2% 0.2% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0	21:00	7	2	က	21	29	45	4				0	0	0	0		38	40
0         0	22:00	2	-	9	15	54	4	12				٥	0	0	0		ස	42
76         6         43         392         1496         1864         681         101         8         0	23:00	٥	0	0	11	48	35	14				0	0	0	0		40	43
1.6% 0.1% 0.9% 8.4% 32.1% 39.9% 14.6% 2.2% 0.2% 0.0% 0.0% 0.0% 0.0% 0.0% 1.1.00 11.00 11.00 11.00 11.00 08:00 11.00 11.00 08:00 11.00 11.00 08:00 11.00 12.0	Total	76	9	43	392	1496	1864	681	1 1			0	0	0	0	1		
11:00 10:00 00:00 11:00 11:00 11:00 08:00 11:00  9 2 2 15 103 167 88 12 5  13:00 21:00 18:00 17:00 18:00 12:00 12:00 12:00 14:00  12 2 6 63 134 189 81 17 1 10 0 0 0 0  191 24 117 869 3138 4129 1708 297 29 4 0 0 0 0 0  1.8% 0.2% 1.1% 8.3% 29.9% 39.3% 16.3% 2.8% 0.3% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0	Percent	1.6%	0.1%	%6.0	8.4%	32.1%	39.9%	14.6%	100			%0.0	%0.0	%0.0	%0.0			
13:00 21:00 18:00 17:00 18:00 12:00 12:00 14:00  12 2 6 63 134 189 81 17 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	AM Peak	11:00	10:00	00:00	11:00	11:00	11:00	11:00								11:00		
13:00 21:00 18:00 17:00 12:00 12:00 12:00 14:00  12 2 6 63 134 189 81 17 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Vol.	Ø	7	2	15	103	167	88	12	5						399		
12 2 6 63 134 189 81 17 1 0 0 0 0 0 0 0 0 191 24 117 869 3138 4129 1708 297 29 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	PM Peak	13:00	21:00	18:00	17:00	18:00	12:00	12:00	12:00	14:00						12:00		
191 24 117 869 3138 4129 1708 297 29 4 0 0 0 0 0 0 1.8% 0.2% 1.1% 8.3% 29.9% 39.3% 16.3% 2.8% 0.3% 0.0% 0.0% 0.0% 0.0% 0.0% 15th Percentile: 29 MPH 50th Percentile: 35 MPH 85th Percentile: 41 MPH 85th Percentile: 41 MPH 85th Percentile: 41 MPH	Vol.	12	2	9	63	134	189	81	17	1				1000		402		
1.8% 0.2% 1.1% 8.3% 29.9% 39.3% 16.3% 2.8% 0.3% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0	Total	191	,	117	869	3138	4129	1708	297	29	4 200	0 00	0 20	0 8	0 80	10506		
15th Percentile: 50th Percentile: 85th Percentile:	Percent	1.8%	%	1.1%	œ;	29.9%	39,3%	16.3%	2.8%	0.3%	%0.0	0.0%	%0.0	%0.0	0.0%			
			₹ [	oth Percenti	 <u>e</u>	29 MPH												
			y y	ith Percenti	<u></u>	35 MPH												
			0 0	th Percenti	 <u>v</u>													

32-41 MPH 6688 63.7% 6167 58.7% 36 MPH

10 MPH Pace Speed:
Number in Pace:
Percent in Pace:
Number of Vehicles > 35 MPH:
Percent of Vehicles > 35 MPH:
Mean Speed(Average):

Stats

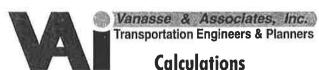
GENERAL BACKGROUND TRAFFIC GROWTH

# Recreation facilities Lymfield, MA

# General Background Traffic Growth

	CITYTOWN	ROUTESTREET	LOCATION	2000	2001	2002	2003	2004	2005	2005	2007	2008	2809	Average Annual Growth Rate
0595 L	LYNNFIELD	RTE, 128 & L 95	1.6 km SOUTH OF PEABODY C.L.	124,469	119,757	122,894	121,150	121,377		97,303	110.646			%t3/1-
7 660S	LYNNFIELD	RTE.128 & I- 95	SOUTH OF WALNUT ST.	129,283					132,094	133,387	131,626	127,558		7665 U
0595 L	LYNNFIELD	RTE I- 95 & 128	1.6 km SOUTH OF PEABODY C.L.	124,469	119,757	122,894	121,150			97,303	110,646			-1.20%
J 6809	LYNNFIELD	RTE.I- 95 & 128	SOUTH OF WALNUT ST.	129,283					132,094	133,387				7.500
													Average Max Min	0.64%

TRIP-GENERATION CALCULATIONS



10 New England Business Center Drive Suite 314 Andover, MA 01810-1066 Office 978-474-8800 Fax 978-688-6508

100			

Job:1	ONLO O	Lynnheld	Kecreation, Pa	IL, & Feladeo	Job Nun	nber: <u>6305</u>	
Location:	Lynn	Reid, HA		Facilities	Date:	1/24/13	
Title:	Trip	beneration	Calculations		Sheet	of	2
Calculated	bv:	ME			Checked	d bv:	

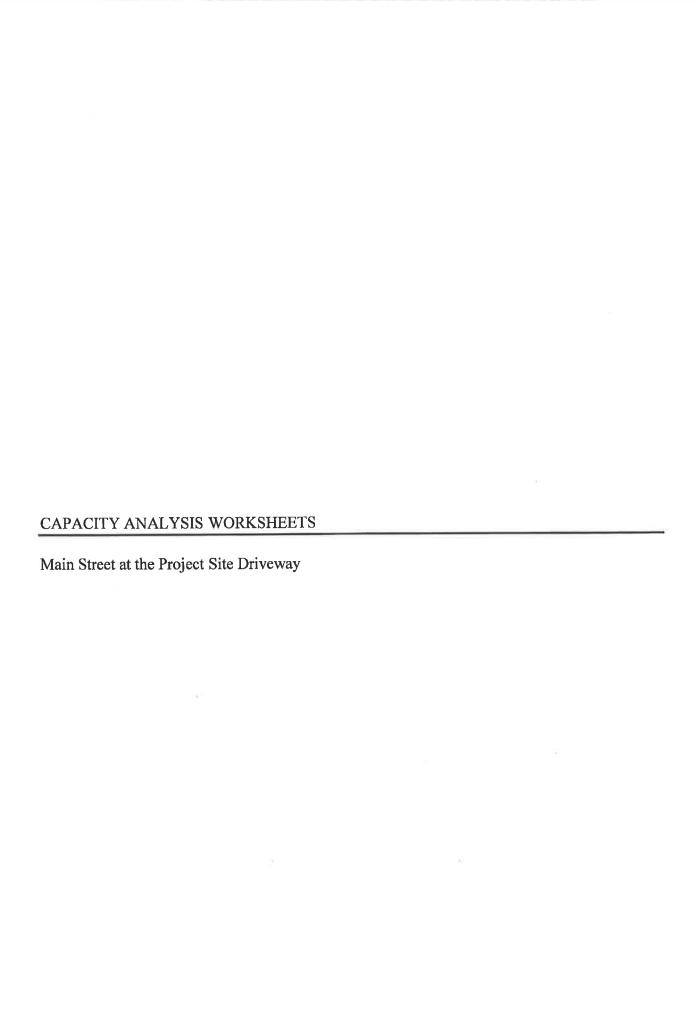
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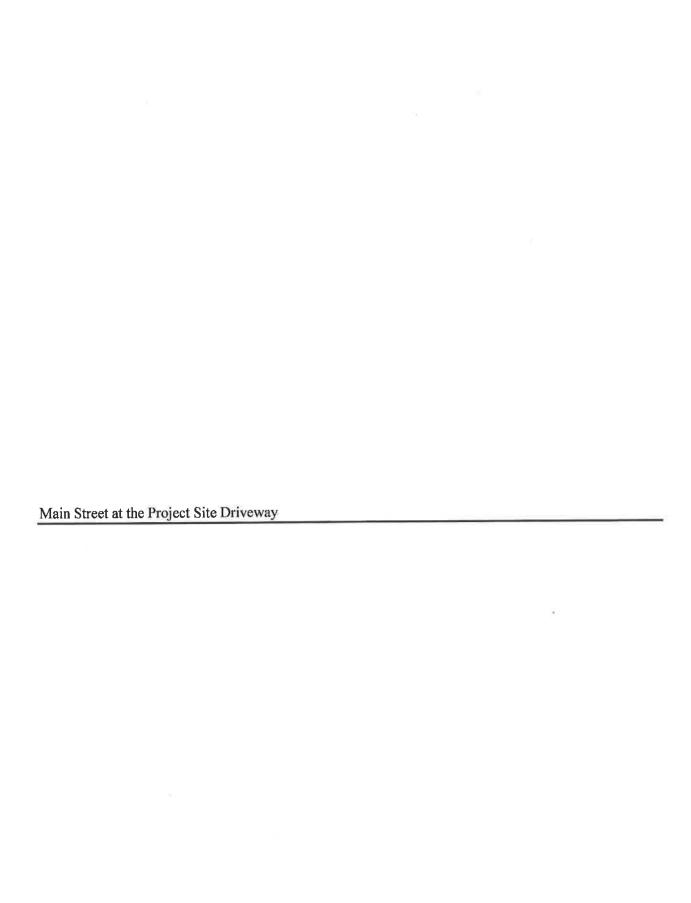
### **Calculations**

10 New England Business Center Drive Suite 314 Andover, MA 01810-1066 Office 978-474-8800 Fax 978-688-6508

Job: Town of Lynnfield Recreation, Park, & Related	Job Number: 6305
Location: Lypofield, MA Facilitie	S Date: 1/24/13
Title: Tro beneration Calculations	of
Calculated by:	Checked by:

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### 2: Main Street & Project Driveway

	1	7	4	<b>†</b>	<b>↓</b>	1	l)
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	NOSSELVE COSSELVE MANAGEMENT
Lane Configurations	W			स	<b>₽</b>		
Volume (vph)	19	172	93	319	183	10	
ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	13	13	14	14	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.879				0.993		
Flt Protected	0.995			0.989			
Satd. Flow (prot)	1629	0	0	1904	1973	0	
Flt Permitted	0.995			0.989			
Satd. Flow (perm)	1629	0	0	1904	1973	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	463			590	575		
Travel Time (s)	10.5			13.4	13.1		
Peak Hour Factor	0.92	0.92	0.97	0.97	0.90	0.90	
Adj. Flow (vph)	21	187	96	329	203	11	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	208	0	0	425	214	0	
Enter Blocked Intersection	Ño	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	0.96	0.96	0.92	0.92	
Turning Speed (mph)	15	9	15			9	
Sign Control	Stop			Free	Free		
Internation Commons	MEDIUS S		OT NO. ALSO THE	200		- 11 ·	

Intersection Summary

Area Type: Control Type: Unsignalized

Other

Intersection Capacity Utilization 53.8%

Analysis Period (min) 15

ICU Level of Service A

2.	Main	Street	&	Project	Driveway
<b>~</b> .	IVICILI	Olicel	v.	1 10100	DITTOTAL

Intersection	WAS OUT	A View	You will be	11 5 81	S. Baro R.	1969000		manife of	
ntersection Delay (sec/veh):	3.8								
Movement	EBL	WINDS NO.	EBR	NBL	NBT	like an Filt	SBT	SBR	
Volume (vph)	19	ESSENT AUGUS	172	93	319		183	10	ATTACA DE LA CAMPACIÓN DE LA C
Conflicting Peds.(#/hr)	0		0	0	010		0	Ö	
Sign Control	Stop		Stop	Free	Free		Free	Free	
Right Turn Channelized	None		None	None	None		None	None	
	0		0	0	140110		110110	0	
Storage Length Median Width	12		U	U	0		0	Ū	
	0%				0%		0%		
Grade (%) Peak Hour Factor	0.92		0.92	0.97	0.97		0.90	0.90	
	0.92		0.52	0.37	2		2	2	
Heavy Vehicles(%) Movement Flow Rate	21		187	96	329		203	11	
Novement Flow Rate Number of Lanes	1		0	0	1		1	Ò	
Number of Lanes			U	U	'			v	
Major/Minor	(1)	GJZ III			Major 1		Major 2	Alexander	
Conflicting Flow Rate - All	730		209	214	0		0	0	
Stage 1	209		:=:		-		-	-	
Stage 2	521		: <b>*</b> /-		_			-	
Follow-up Headway	3.518		3.318	2.218	-			-	
Pot Capacity-1 Maneuver	389		831	1356			*	*	
Stage 1	826		43	- 1	2		*	.060	
Stage 2	596			_	2			4	
Time blocked-Platoon(%)	0		0	0	-				
Mov Capacity-1 Maneuver	355		831	1356	-			-	
Mov Capacity-2 Maneuver	355		-	-				-	
Stage 1	826		-	_				-	
Stage 2	544			-				*	
Stage 2	V44								
Approach	EB		3.5	NB	S. Francisco		SB		MATTER ST
HCM Control Delay (s)	11.8			1.8			0		
HCM LOS	В			Α			Α		
0.2002	ning design	MDI	NDT	EDI 54	SBT	SBR		an Sakabara	
Lane		NBL	NBT	EBLn1	301	ODIT		100	
Capacity (vph)		7.05=		733					
HCM Control Delay (s)		7.857		11.8	-	-			
HCM Lane VC Ratio		0.071	-	0.283	**	*			
HCM Lane LOS		A 0.228		B 1,165	-	*			
HCM 95th Percentile Queue (	1 1	0.000							

### 2: Main Street & Project Driveway

	1	7	4	<b>†</b>	<b>↓</b>	1	
Lane Group	EBL	EBR	NBL	NBT.	SBT	SBR	
Lane Configurations	Ϋ́			स	ĵ⇒		
Volume (vph)	13	113	151	205	222	17	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	13	13	14	14	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	0.879				0.990		
FIt Protected	0.995			0.979			
Satd. Flow (prot)	1629	0	0	1884	1967	0	
Flt Permitted	0,995			0.979			
Satd. Flow (perm)	1629	0	0	1884	1967	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	463			590	575		
Travel Time (s)	10.5			13.4	13.1		
Peak Hour Factor	0.92	0.92	0.93	0.93	0.77	0.77	
Adj. Flow (vph)	14	123	162	220	288	22	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	137	0	0	382	310	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	12			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	0.96	0.96	0.92	0.92	
Turning Speed (mph)	15	9	15			9	
Sign Control	Stop			Free	Free		
Internation Cummuns	WINDOWS IN	12002577000	THE REAL PROPERTY.	A MOSSIEW	molton and the	SHEERICA	

Intersection Summary

Area Type: Other Control Type: Unsignalized

Intersection Capacity Utilization 49.6%

Analysis Period (min) 15

ICU Level of Service A

Intersection	I Tulis		Cole prive	8.00	t Assis		Server St	project in		
Intersection Delay (sec/veh):	3.6									
Movement	EBL		EBR	NBL	NBT		SBT	SBR		
Volume (vph)	13		113	151	205		222	17		
Conflicting Peds.(#/hr)	0		0	0	0		0	0		
Sign Control	Stop		Stop	Free	Free		Free	Free		
Right Turn Channelized	None		None	None	None		None	None		
Storage Length	0		0	0				0		
Median Width	12				0		0			
Grade (%)	0%				0%		0%			
Peak Hour Factor	0.92		0.92	0.93	0.93		0.77	0.77		
Heavy Vehicles(%)	2		2	2	2		2	2		
Movement Flow Rate	14		123	162	220		288	22		
Number of Lanes	1		0	0	1		1	0		
Major/Minor	3149	ALCOHOL:		19-81-3	Major 1		Major 2	12767/8		A DIA
Conflicting Flow Rate - All	843		299	310	0		0	0	,	
Stage 1	299		-		-		-	-		
Stage 2	544		175		_		-			
Follow-up Headway	3.518		3.318	2.218	180		-	-		
Pot Capacity-1 Maneuver	334		741	1250			-	-		
Stage 1	752		-	•	-		196	•		
Stage 2	582			-	-		5#5	390		
Time blocked-Platoon(%)	0		0	0	-		-	(#S)		
Mov Capacity-1 Maneuver	285		741	1250	-		=	1 <u>14</u> 7		
Mov Capacity-2 Maneuver	285		=		-		-			
Stage 1	752		*	70	-					
Stage 2	496		-	0.00	-		: <del></del>	3.00		
Approach	EB	S. 22.0	K IN	NB	THE REAL PROPERTY.	12 SE 2 C	SB		A PRODUCTION OF	
HCM Control Delay (s)	12.2			3.5			0			
HCM LOS	В			Α			Α			
Lane	ST 15, 74	NBL	NBT	EBLn1	SBT	SBR			FERREIM	DISEAS.
Capacity (vph)				636						
HCM Control Delay (s)		8.309		12.2		-				
HCM Lane VC Ratio		0.13	-	0.215	070	_				
HCM Lane LOS		Α	-	В	700	-				
HCM 95th Percentile Queue	(veh)	0.446	-	0.813	( 60	-				
	,,									

5 5	×		

# Enclosure 9 Development Cost Estimates

# SCHEMATIC COST ESTIMATE - MAIN STREET RECREATION PARCEL - OPTION 1 LYNNFIELD, MA MASTER PLAN

This cost estimate reflects the improvements proposed at the Main Street Recreation Complex, including one (1) multipurpose synthetic turf game field, two (2) multipurpose natural turf fields,

EM	parking lots, access roadwa		QUANTITY			cos			REMARKS
TIVE F	I RECREATION AREA (NORTH)								
	0 10 10				A Land			\$ 87,432.93	
a	General Conditions Bonds and Insurance (2%)	LS	1	\$	67,432.93	\$	67,432.93	\$ 67,432.93	
b	Mobilization/Demobilization	LS	1	\$			20,000.00		
	Erosion Control	150	W. F					\$ 74,700.00	
а	Haybales and Silt Fence	LF	8300	\$	9.00	\$	74,700,00	May/asia	*assume length of buffer zone
	Describbles			-				\$ 30,000.00	
а	Demolition Misc, Demolition	LS	1		\$30,000.00	\$	30,000.00	3 30,000.00	
-	Wilder Demonition				400,000.00		001000100		
	Synthetic Turf Field Construction - Field 1 (w / Lights)			14		79.1		\$ 1,087,435.00	
a	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1800	\$	12.00	\$	21,600,00		
b	Prepare sub-base, shape and compact	SY	9,700	\$	2.25	\$	21,825.00		
c d	Drainage Geotextile Separation Layer	SY	9,700	\$	2.00	\$	19,400.00		
e e	10" Perf, HDPE	LF	990	\$	25.00	\$	24,750.00	THE RESTA	*estimated quantity at schematic level
f	Flat panel drains	LF	2800	\$	4.00	\$	11,200.00		*estimated quantity at schematic level
g	Cleanouts (Nyloplast CB's)	EA	6	\$	1,600,00	\$	9,600.00		"estimated quantity at schematic level
h	Field Base						N. S. A.		
1	Crushed Stone Base under Field (8")	CY	2200	\$			79,200,00		
k	Crushed Stone Base under Field (2") Concrete	CY	540	\$	37.00	\$	19,980.00		
n	Cast in place Concrete Curb without Trench Drain	LF	1220	\$	32,00	\$	39,040.00		
n	Field Fencing			Ť	32,00				
0	4' High Perimeter Fence	LF	1220	\$	40,00	\$	48,800,00		
р	12' Wide Gate	EA	2	\$	1,725.00	\$	3,450.00		
q	4' Pedestrian Gale	EA	4	\$	560.00	\$	2,240.00		
r	Water Supply		- 4	-	7,000.00	•	7,000,00		
t t	Water Cannon Water Cannon Connection	EA EA	4	\$	1,500.00	\$	7,000,00 6,000,00		
u u	Water Cannon Connection  Water Line	LF	1150	\$	22.00	\$	25,300.00		*estimated quantity at schematic level
v	Field Surfacing		1100	-	LLIGO		20,000,00		
w	Filled-Turf installed	SF	87,400	\$	4.25	\$	371,450.00		
x	Turf striping	Sport	4	\$	7,000.00	\$	28,000.00		
У	Equipment			-		100	all Parket		
Z	Scoreboard	LS	1	\$	25,000.00	\$	25,000,00		
aa	Goals	PR LS	1	\$	3,400.00 \$30,000.00	\$	13,600.00 \$30,000.00		*does not include electric service to parcel
cc bb	Site Electrical (connection of system) MUSCO Athletic Field Lighting System	Pole	4		\$70,000.00		\$280,000.00	Control of the contro	does not include execute service to parcer
	Natural Turf Field - Field 2							\$ 341,142.50	
а	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1500	\$	12.00	\$	18,000.00		
b	Prepare sub-base, shape and compact	SY	8,950	\$	2.25	\$	20,137,50		
С	Drainage								
d	Geolextile Separation Layer	SY	8,950	\$	2.00	\$	17,900.00		
e	12" Perf. HDPE	LF	930	\$	28.00	\$	26,040,00		*estimated quantity at schematic level
f	Flat panel drains Cleanouts (Nyloplast CB's)	LF EA	2000	\$	4.00 1,600.00	\$	9,600.00		*estimated quantity at schematic level
g h	Field Base		- 0	1	1,000,00	Ť	3,000.00		
i	Crushed Stone Base under Field (4")	CY	1000	\$	36,00	\$	36,000.00		
Ĺ	Place and amend root zone materials (8")	CY	2000	\$		\$	54,000,00		
k	Irrigation								
1	Water line	LF	600	\$	22.00		13,200.00		*estimated quantity at schematic level
m.	Irrigation System and controller	LS	1	\$	30,000.00	\$	30,000.00		*imigation well / pump separate
0	Field Surfacing Seed athletic field mix and fine grade	SF	80,500	\$	0.35	S	28,175,00		
p p	Turf Establishment Requirements	LS	1	\$	8,000,00	_	8,000.00		
q	Field Fencing			1	2,2000				
r	4' High Perimeter Fence	LF	1160	\$		\$	46,400,00		
S	12' Wide Gate	EA	2	\$	1,725.00	\$	3,450.00		
l	4' Pedestrian Gate	EA	_4	\$	560.00	S	2,240,00		
u	Equipment	1.0	4	\$	20,000.00	0	20,000.00		
v	Scoreboard	LS	1	3	20,000,00	\$	20,000.00		
	Natural Turf Field - Field 3							\$ 341,142.50	
a	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1500	\$	12.00	\$	18,000.00		
b	Prepare sub-base, shape and compact	SY	8,950	\$	2,25	\$	20,137,50	Dispute I. L.	
С	Drainage							- V	
d	Geotextile Separation Layer	SY	8,950	\$			17,900,00		
е	12" Perf, HDPE	LF	930	\$	28.00	\$	26,040.00		*estimated quantity at schematic level
f	Flat panel drains	LF	2000	\$	4.00	\$	8,000.00		*estimated quantity at schematic level
g b	Cleanouts (Nyloplast CB's) Field Base	EA	6	1	1,600.00	\$	9,600.00		
hi	Crushed Stone Base under Field (4")	CY	1000	\$	36.00	\$	36,000.00		
_	Place and amend root zone materials (8")	CY	2000	\$	27,00	\$	54,000.00		
i				-		_			
k	Irrigation						1111		

m	Irrigation System and controller	LS	1	\$	30,000.00	\$	30,000.00		*irrigation well / pump separate
п	Field Surfacing								
0	Seed athletic field mix and fine grade	SF	80,500	\$	0.35	_	28,175,00		
р.	Turf Establishment Requirements	LS	1	\$	8,000.00	\$	8,000.00		
q	Field Fencing			_	40.00		10.100.00		
Г	4' High Perimeter Fence	LF EA	1160	\$	40.00 1.725.00	\$	46,400,00 3,450,00		
S	12' Wide Gate		4	\$	560.00	\$	2,240.00		
t	4' Pedestrian Gate	EA	4	2	200-00	9	2,240,00		
u	Equipment Scoreboard	LS	1	\$	20,000.00	\$	20,000.00		
	Scoreboard	20			20,000,00	-	20,000,00		TAX IV SECTION STREET
7	Amenities Building							\$ 396,500.00	
а	1,800 SF building (bare concession, storage, restrooms)	SF	1800	\$	175.00	s	315,000.00		
b	Underground Electrical Service	LF	1500	\$	21.00	\$	31,500.00		*estimated quantity at schematic level
C	Water Service	LF	1000	\$	50.00	S	50,000.00	THE PERSON	*estimated quantity at schemalic level
8	Irrigation Well / Pump	14						\$ 14,000.00	
8:	Irrigation Well	LF	100	\$	75.00	\$	7,500.00		
ь	Pump at Irrigation well	EA	1	\$	6,500.00	\$	6,500.00		
								\$ 10,000.00	
9	Septic System Septic Tank w/ D-Box and Leaching Field	EA	LS	\$	10,000.00	\$	10,000.00	\$ 10,000.00	
a	Septic Tank W/ D-Box and Leaching Field	1	- 13	9	10,000.00	0	10,000,00		
10	Spectator Seating							\$ 22,500.00	
а	100-seat mobile bleachers	EA	3	\$	7,500.00	\$	22,500.00		
			118				IIO II		
11	Walkways / Access Drives	1		1				\$ 144,378.75	
a	Prepare sub-base, shape and compact	SY	2535 2535	\$	2.25 8.00		5,703.75 20,280.00		
b c	Gravel Base (8" base) Pavement (1,5" Binder course and 1,5" Wearing Course)	SY	2535	\$	22.00		55,770,00	V SEE A BUILT	
d	Site Lighting Bollards	EA	15	\$	4,175.00		62,625.00		*estimated quantity at schematic level
						10			
12	Main Parking Lot	CV	1050	\$	12.00	\$	12,600.00	\$ 334,900.00	
a c	Strip and haul topsoil / organics (assume 6" per test pits)  Gravel Base (8" base)	CY	1050 6300	\$	8.00		50,400,00		
d	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	6300	\$	25.00		157,500.00		
е	Signage	LS		\$	1,500.00	\$	1,500,00		
_ f	Drainage Control of the Control of t	EA		\$	2,800.00	•	14,000.00		
g h	Catch Basins / Manholes Subsurface Detention System	LS	5	\$	52,000.00		52,000.00		
-	12* RCP	LF	270	\$	50.00		13,500.00	real districts	
	Parking lot Lighting	pole	8	\$	4,175.00		33,400.00		
Je Kalifi								* 00.050.00	
13	Northern Parking Lot & Access Drive	CY	250	\$	12.00	\$	3,000.00	\$ 80,650.00	
c	Strip and haul topsoil / organics (assume 6" per test pits)  Gravel Base (8" base)	SY	1500	\$	8.00	\$	12,000.00		
d	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	1500	\$	25.00		37,500.00		
е	Signage	LS	1	\$	1,500.00	\$	1,500,00		
f	Drainage		1	0	2,800.00	6	2,800.00		
9	Catch Basins / Manholes Subsurface Detention System	LS	0	\$	52,000.00		2,000.00		
h i	12" RCP	LF	70	\$	50.00		3,500.00		
	Bioretention Ponds	LS	1	\$	12,000.00	\$	12,000.00		
k	Parking lot Lighting	pole	2	\$	4,175.00	\$	8,350.00	10000	
45	Double of Barbins Lab							\$ 151,730.00	
15 a	Southern Parking Lot Strip and haul topsoil / organics (assume 6" per test pits)	CY	470	\$	12.00	\$	5,640,00	101/100100	
b b	Gravel Base (8" base)	SY	2780	\$	8.00	\$	22,240.00		
С	Pavement (1,5" Binder course and 1,5" Wearing Course)	SY	2780	\$	25.00	\$	69,500.00		
d	Signage	LS	_1	\$	1,500.00	\$	1,500.00		
e f	Drainage Catch Basins / Manholes	EA	3	ŝ	2,800.00	ŝ	8,400.00		
	Subsurface Detention System	LS	0	\$	52,000.00		0,100,00	E STATE	
h	12" RCP	LF	45	\$	50.00	\$	2,250.00		
	10" HDPE Perf. Pipe	LF	220	\$	25.00		5,500.00		
k	Bioretention Ponds Parking lot Lighting	LS	4	\$	20,000.00 4,175.00		16,700.00		
	r arking for Enginering	pole		Ĭ	311.05.00	Í	, 5,7 00,00		
14	Trails at Recreation Park		1 190	15. =				\$ 96,750.00	
а	Prepare sub-base, shape and compact	SY	3000	\$	2.25 8.00		6,750.00		
С	Gravel Base (8" base) Pavement (1,5" Binder course and 1,5" Wearing Course)	SY	3000	\$	22.00		24,000.00 66,000.00		
	are more than period, could did no fround doubted		5500				7,5,5,5		
15	Landscaping							\$ 36,000.00	
a	Landscaping Planting Areas (entrances)	EA	2	\$	8,000.00	\$	16,000.00 20,000.00		*quantity estimated at schematic level
ь	Loom and Seed Areas (including at demolished bleacher)	LS	1	\$	20,000.00	2	20,000,00		quantity estimated at strictfiality level
16	Site Drainage						7.00	\$ 47,200.00	أندون والمكروف والمرابع والمرابع
а	12" HDPE Pipe	LF	600	\$	28.00		16,800.00		*quantity estimated at schematic level
b	Calch Basins / Manholes	EA	- 8	\$	2,800.00		22,400.00		*quantity estimated at schematic level
C	Nyloplast Drain Structures	EA	5	\$	1,600.00	\$	8,000.00		*quantity estimated at schematic level
17			100		100			\$ 41,617.50	
11.6	Basketball Court			1.	40.00	\$	1 200 00	· · · · · · · · · · · · · · · · · · ·	
17 a	Basketball Court Strip and haul topsoil / organics (assume 6" per test pits)	CY	100	\$	12.00	Ψ	1,200.00		
	Strip and haul topsoil / organics (assume 6" per test pits) Prepare sub-base, shape and compact	SY	470	\$	2,25	\$	1,057.50		
a b c	Strip and haul topsoil / organics (assume 6" per test pits) Prepare sub-base, shape and compact Gravel Base (8" base)	SY	470 470	\$	2,25 8.00	\$	1,057.50 3,760.00		
a b c d	Strip and haul topsoil / organics (assume 6" per test pits) Prepare sub-base, shape and compact Gravel Base (8" base) Pavement (1.5" Binder course and 1.5" Wearing Course)	SY SY SY	470 470 470	\$ \$ \$	2,25 8.00 25.00	\$ \$ \$	1,057.50 3,760.00 11,750.00		
a b c	Strip and haul topsoil / organics (assume 6" per test pits) Prepare sub-base, shape and compact Gravel Base (8" base)	SY	470 470	\$	2,25 8.00	\$	1,057.50 3,760.00		

							- The Later	
18	Playgrounds				, LOY	M II Jeji	\$ 110,000.00	
а	Small Play Area	LS	1	\$ 40,000.00		40,000.00		
b	Large Play Area	LS	1	\$ 70,000.00	\$	70,000.00		
19	Seating Areas				101		\$ 11,000.00	
а	Benches	LF	20	\$ 400.00	\$	8,000.00	M	
b	Picnic Tables	EA	15	\$ 200.00	\$	3,000.00		
				ACTIVE DECDEAT	CION AD	EA TOTAL		
		-		ACTIVE RECREAT Subtotal:	ION AR	EA TOTAL	\$ 3,459,079,18	
				Soft Costs (7%)			\$ 242,135.54	
				10% Contingency			\$ 345,907.92	
				TOTAL	_		\$ 4,047,122,63	
DO A DIAL								
ROADWA	Entrance Roadway			V-S-	-0.54	100	\$ 1,249,700.00	
8	Clearcutting	AC	13.2	\$7,000.00	\$	92,400,00	<del></del>	Clearcut Active Sile and Roadway
b	Cut & Fill	CY	37100	\$8.00	_	296,800.00		Excavate/backfill with dozer & haul on site material
c	Net Cut Excavation & haul	CY	900	\$4.00		3,600,00		Net cut excavation and haul
d	Strip and haut topsoil / organics (assume 6")	CY	2600	\$ 12,00	\$	31,200,00		
е	Gravel Base (12" base)	SY	5300	\$ 16.00		84,800.00		
	Pavement (1,5" Binder course and 1.5" Wearing Course)	SY	5300	\$ 25.00 \$ 3,000.00		3,000.00		
g h	Signage Drainage	LS	1	\$ 3,000.00	φ	3,000.00		
7	Catch Basins / Manholes	EA	12	\$ 2,800,00	\$	33,600.00	US EXPLESS	*quantity estimated at schematic level
	12" RCP	LF	1500	\$ 50.00	\$	75,000.00	DE ALE S	*quantity estimated at schematic level
k	Stormwater Management	LS	1	\$ 80,000.00		80,000,00		
l m	Parking lot Lighting Walls / Culverts / Wetland Crossings	LS	16	\$ 4,175,00 \$ 100,000.00		66,800.00		
n	Electric Service to Site	LS	1	\$ 250,000.00		250,000.00	IV.	
	ELOUIS OF FIGURE				1 21 9			
				ROADWAY TOTAL	L:		4 0 10 700 70	
			-	Subtotal:			\$ 1,249,700.00 \$ 124,970.00	
				10% Contingency			\$ 1,374,670.00	
					100			Determines The Karawa
PASSIVE	RECREATION AREA						470.000.00	
1	Parking Area & Access Drive			87.000.00	1	7.000.00	\$ 179,980.00	
a	Clearcutting	CY	540	\$7,000.00		7,000.00 6,480.00		
b	Strip and haul topsoil / organics (assume 6" per test pils) Gravel Base (8" base)	SY	3200	\$ 8.00		25,600.00		
- d	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	3200	\$ 22.00		70,400.00		
e	Signage	LS	1	\$ 1,500.00	\$	1,500.00		The second secon
f	Drainage				70	40 000 00		Paragraphy and paragraphy and a physical based
g h	Catch Basins / Mariholes 12" RCP	EA LF	6 50	\$ 2,800.00 \$ 50.00		16,800.00 2,500.00		"quantity estimated at schematic level "quantity estimated at schematic level
-	12" HDPE Pipe	LF	300	\$ 50.00		15,000.00		"quantity estimated at schematic level
	Bioretention Ponds	LS	1	\$ 18,000.00		18,000.00		
k'	Parking lot Lighting	pole	4	\$ 4,175.00	\$	16,700.00		
0	Total Manager				1		\$ 237,900.00	
3	Trail Network Clearcutting	AC	1.2	\$7,000.00	s	8,400.00	201,000.00	Committee of the second
b	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1500	\$ 12.00		18,000.00		
C	Prepare sub-base, shape and compact	SY	6000	\$ 2.25	\$	13,500.00		
d	Gravel Base (8" base)	SY	6000	\$ 8.00		48,000.00		
e	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	6000	\$ 22.00		132,000.00	CO PRATE	
f	Gravel Surface	SY	3000	\$ 6.00	φ.	18,000.00		
3	Dog Park						\$ 143,100.00	
а	Clearcutting	AC	1	\$7,000.00		7,000.00		
b	Strip and haul topsoil / organics (assume 6" per test pils)	CY	650	\$ 12.00		7,800.00		
C	Prepare sub-base, shape and compact	SY	4000	\$ 2.25		9,000.00		
d	Gravel Base (8" base) Crushed Stone surface	SY	4000 600	\$ 8.00 \$ 22.00		32,000.00 13,200.00		
e f	Walkways	SY	270	\$ 30.00		8,100.00		
g	Accessories / stones / benches	LS	1	\$ 10,000.00	\$	10,000.00	No. of the Land	
h	6' chain link fence	LF	800	\$ 45.00		36,000.00		
I	Accessories / stones / benches landscaping	LS	1	\$ 20,000.00	\$	20,000.00		
4	Open Space						\$ 92,200.00	
a	Clearcutting & Demolition	AC	1.2	\$8,500.00		10,200,00	S	
b	Prepare sub-base, shape and compact	SY	5500	\$ 4.00		22,000.00		
C	Earthwork	LS	1 1	\$ 18,000.00		18,000.00		
d	Loam and Seed	LS	1	\$ 22,000.00 \$ 20,000.00		22,000.00		
e	Landscaping	Lo		20,000.00	Ti II	24,000,00	علته واستعمارا	
	والمناب والمناب والمناب والمناب والمناب والمناب			PASSIVE RECREA	ATION A	AREA TOTA		
200				Subtotal:			\$ 653,180.00	
-		11.0		Soft Costs (7%)			\$ 45,722.60	
				10% Contingency			\$ 65,318.00 \$ 764,220.60	

## SCHEMATIC COST ESTIMATE - MAIN STREET RECREATION PARCEL - OPTION 2 LYNNFIELD, MA MASTER PLAN

This cost estimate reflects the improvements proposed at the Main Street Recreation Complex under Option 2, including two (2) multipurpose synthetic turf game fields, one (1) multipurpose natural turf field, parking lots, access roadway, passive recreation opportunities, pedestrian circulation routes, and related amenities.

M	DESCRIPTION	UNIT	QUANTITY	UNIT COST	COST	TOTAL COST	REMARKS
TIVE 6	RECREATION AREA (NORTH)						
)	General Conditions					\$ 97,031,48	
a	Bonds and Insurance (2%)	LS	1	\$ 77,031.48	\$ 77,031,48		
b	Mobilization/Demobilization	LS	1	\$ 20,000.00	\$ 20,000.00		
	Erosion Control					\$ 74,700,00	
3	Haybales and Sill Fence	LF	8300	\$ 9.00	\$ 74,700.00	74,180,00	*assume length of buffer zone
	Demolition					\$ 100,000.00	
1	Misc, Demolition	LS	1	\$30,000.00	\$ 30,000.00 \$ 70,000.00		
)	Clearcutting	AC	10	\$7,000.00	\$ 70,000.00		
	Synthetic Turf Field Construction - Field 1 (w / Lights)					\$ 1,087,435.00	
	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1800	\$ 12,00	\$ 21,600,00		
	Prepare sub-base, shape and compact	SY	9,700	\$ 2,25	\$ 21,825,00		
=	Drainage Geolextile Separation Layer	SY	9,700	\$ 2,00	\$ 19,400.00		
	10" Perf, HDPE	LF	990	\$ 25,00	\$ 24,750.00		*estimated quantity at schematic level
	Flat panel drains	LF	2800	\$ 4,00	\$ 11,200.00		*estimated quantity at schematic level
	Cleanouts (Nyloplast CB's)	EA	6	\$ 1,600,00	\$ 9,600.00		*estimated quantity at schematic level
	Field Base Crushed Stone Base under Field (8")	CY	2200	\$ 36,00	\$ 79,200.00		
	Crushed Stone Base under Field (8") Crushed Stone Base under Field (2")	CY	540	\$ 37,00	\$ 19,980.00	NAME OF BRIDE	
	Concrete						
	Cast in place Concrete Curb without Trench Drain	LF	1220	\$ 32,00	\$ 39,040.00		
	Field Fencing	LF	1220	\$ 40.00	\$ 48,800.00		
	4' High Perimeter Fence 12' Wide Gate	EA	1220	\$ 40.00 \$ 1,725.00	\$ 48,800,00 \$ 3,450,00		
	4' Pedestrian Gate	EA	4	\$ 560.00	\$ 2,240.00		
	Water Supply						
	Water Cannon	EA	1	\$ 7,000.00	\$ 7,000.00		
	Water Cannon Connection	EA LF	1150	\$ 1,500.00 \$ 22.00	\$ 6,000.00 \$ 25,300.00		*estimated quantity at schematic level
=	Water Line Field Surfacing	L	1130	9 22.00	23,300.00		escribica quarany at suremania levo.
	Filled-Turf installed	SF	87,400	\$ 4.25	\$ 371,450.00		
	Turf striping	Sport	4	\$ 7,000.00	\$ 28,000.00		
/	Equipment	LS	1	\$ 25,000,00	\$ 25,000,00		
a a	Scoreboard Goals	PR	1 4	\$ 3,400.00	\$ 13,600.00		
b.	Site Electrical (connection of system)	LS	1	\$30,000.00	\$30,000 00		"does not include electric service to parcel
DC .	MUSCO Athletic Field Lighting System	Pole	4	\$70,000.00	\$280,000.00		
	Synthetic Turf Field Construction - Field 2					\$ 751,070.00	
а	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1600	\$ 12.00	\$ 19,200.00	4 131,010.00	
0	Prepare sub-base, shape and compact	SY	9,330	\$ 2.25	\$ 20,992.50		
;	Drainage				THE VICE S		
	Geolextile Separation Layer	SY	9,330	\$ 2.00	\$ 18,660.00		
	10" Perf, HDPE Flat panel drains	LF LF	960 2600	\$ 25.00 \$ 4.00	\$ 24,000.00 \$ 10,400.00		*estimated quantity at schematic level *estimated quantity at schematic level
,	Cleanouts (Nyloplast CB's)	EA	6	\$ 1,600.00	\$ 9,600.00		*estimated quantity at schematic level
1	Field Base						
	Crushed Stone Base under Field (8")	CY	2100	\$ 36.00	\$ 75,600.00		
	Crushed Stone Base under Field (2")	CY	520	\$ 37,00	\$ 19,240.00		
1	Concrete Cast in place Concrete Curb without Trench Drain	LF	1200	\$ 32.00	\$ 38,400.00		
	Field Fencing						
	4' High Perimeter Fence	LF	1190	\$ 40.00	\$ 47,600.00		
	12' Wide Gate	EA	2	\$ 1,725.00	\$ 3,450.00		
	4' Pedestrian Gate Water Supply	EA	4	\$ 560,00	\$ 2,240,00		
	Water Supply Water Cannon	EA	1	\$ 7,000.00	\$ 7,000.00	2m. 00-00	
	Water Cannon Connection	EA	4	\$ 1,500,00	\$ 6,000.00		
	Waler Line	LF	1150	\$ 22.00	\$ 25,300.00		*estimated quantity at schematic level
,	Field Surfacing Filled-Turf installed	SF	83,950	\$ 4.25	\$ 356,787.50		
v <	Turf striping	Sport	4	\$ 7,000.00	\$ 28,000.00		
	Equipment					Jack LAS	
	Scoreboard	LS	1	\$ 25,000.00	\$ 25,000.00		
3	Goals	PR	4	\$ 3,400,00	\$ 13,600,00		
-	Natural Turf Field - Field 3					\$ 341,142,50	
	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1500	\$ 12.00	\$ 18,000.00	. 041,142.00	
	Prepare sub-base, shape and compact	SY	8,950	\$ 2,25	\$ 20,137.50		
	Drainage_						
)	Geotextile Separation Layer	SY	8,950 930	\$ 2,00 \$ 28,00	\$ 17,900.00 \$ 26,040.00		Pactimolog quantity of a share Falauri
	12" Perf. HDPE Flat panel drains	LF LF	2000	\$ 28.00 \$ 4.00	\$ 26,040,00		*estimated quantity at schematic level *estimated quantity at schematic level
7	Cleanouts (Nyloplast CB's)	EA	6	\$ 1,600.00	\$ 9,600.00		at societilane level
	Field Base						
	Crushed Stone Base under Field (4*)	CY	1000	\$ 36,00	\$ 36,000.00		
		CY	2000	\$ 27.00	\$ 54,000,00		
	Place and amend root zone materials (8")	CT	2000				
	Place and amend root zone materials (8")  Irrigation  Water line	LF	600	\$ 22.00	\$ 13,200.00		*estimated quantity at schematic level

				-					
n:	Field Surfacing			1			00.4===		
0:	Seed athletic field mix and fine grade	SF	80,500	\$		\$	28,175.00		
p:	Turf Establishment Requirements	LS	1	\$	8,000,00	\$	8,000,00		
q	Field Fencing			1					
r	4' High Perimeter Fence	LF	1160	\$		\$	46,400.00		
S	12' Wide Gale	EA	2	\$		\$	3,450.00		
	4' Pedestrian Gate	EA	4	\$	560,00	\$	2,240.00		
u	Equipment								
V	Scoreboard	LS	1	\$	20,000.00	\$	20,000.00		
THE NAME									
7	Amenities Building	1100					إلىت درو	\$ 396,500.00	
а	1,800 SF building (bare concession, storage, restrooms)	SF	1800	\$		\$	315,000.00		
b	Underground Electrical Service	LF	1500	\$		\$	31,500.00		*estimated quantity at schematic level
С	Water Service	LF	1000	\$	50.00	\$	50,000.00		*estimated quantity at schematic level
00					In March				
В	Irrigation Well / Pump		T less	DV.	IT IN THE	-		\$ 14,000,00	
а	Irrigation Well	LF	100	\$	75.00	\$	7,500.00		
b	Pump at Irrigation well	EA	1	\$	6,500.00	\$	6,500.00	100000000000000000000000000000000000000	
	I SO I WAS IN THE RESERVE OF THE PARTY OF TH			100				ALESTA ALA	
9	Septic System					10		\$ 10,000,00	
8	Septic Tank w/ D-Box and Leaching Field	EA	LS	\$	10,000.00	\$	10,000.00		
			0 1			100			
10	Spectator Seating	1.7				111		\$ 22,500,00	
ă	100-seat mobile bleachers	EA	3	\$	7,500.00	\$	22,500.00		
	Market and the second s							444.070.75	
11	Walkways / Access Drives	SY	2535	S	2.25	\$	5,703.75	\$ 144,378,75	
a b	Prepare sub-base, shape and compact Gravel Base (8" base)	SY	2535	\$			20,280.00		
c	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	2535	\$	22.00	\$	55,770.00		
d	Site Lighting Bollards	EA	15	\$	4,175.00	\$	62,625,00		*estimated quantity at schematic level
				J. Pilin		150			
12	Main Parking Lot	11200			VINCE IN	100	Mark Control	\$ 334,900.00	
а	Strip and haul topsoil / organics (assume 6" per test pits)	CY	1050	\$	12,00	\$	12,600.00		
C.	Gravel Base (8" base) Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	6300 6300	\$	8.00 25.00	\$	50,400.00 157,500.00		
d e	Signage	LS	1	\$	1,500.00	\$	1,500.00		
f	Drainage				1,000,00	1156			
9	Calch Basins / Manholes	EA	5	\$	2,800.00		14,000,00		
h	Subsurface Detention System	LS	11	\$	52,000_00	\$	52,000.00		
	12" RCP	LF	270	\$	50.00	\$	13,500.00		CARL CARL CARL CARL
	Parking lot Lighting	pole	8	\$	4,175.00	\$	33,400.00		
40	Markey Barbard Ad Assess Britis							\$ 80,650.00	
13 a	Northern Parking Lot & Access Drive Strip and haul topsoil / organics (assume 6" per test pits)	CY	250	s	12.00	s	3,000.00	3 00,000.00	NI STATE OF THE ST
C	Gravel Base (8" base)	SY	1500	\$	8.00	\$	12,000.00		
d	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	1500	\$	25.00	\$	37,500.00		THE RESERVE OF THE PARTY OF THE
е	Signage	LS	1	\$	1,500.00	\$	1,500.00		
f	Drainage			100					
g	Catch Basins / Manholes	EA	_1	\$		\$	2,800.00		
h	Subsurface Detention System	LS	0	\$	52,000.00	\$	9 500 00	U12 U R 31 -33	
- 1	12" RCP Bioretention Ponds	LF	70	\$	50.00 12,000.00	2	3,500.00 12,000.00		
k	Parking lot Lighting	pole	2	\$		\$	8,350.00		
,,		Pois	3.0						De Total Administration of Earlies and Earlies
15	Southern Parking Lot							\$ 151,730.00	
а	Strip and haul topsoil / organics (assume 6" per test pits)	CY	470	\$	12.00	\$	5,640.00		
b	Gravel Base (8" base)	SY	2780	\$	8.00	\$	22,240.00		
c	Pavement (1.5" Binder course and 1.5" Wearing Course)	LS	2780	\$	25.00 1,500.00	\$	69,500.00 1,500.00		
d e	Signage	LS	1	-	1,500.00	9	1,000.00		
e	Drainage Catch Basins / Manholes	EA	3	\$	2,800.00	S	8,400.00		
g	Subsurface Detention System	LS	0	\$		\$	0,400.00		
h	12" RCP	LF	45	\$	50.00	\$	2,250.00		
j	10" HDPE Perf. Pipe	LF	220	\$	25.00	\$	5,500.00		
_i_	Bioretention Ponds	LS	111	\$	20,000.00	\$	20,000.00		
k	Parking lot Lighting	pole	4	\$	4,175.00	\$	16,700.00		
	To the second se			1.0				¢ 00.750.00	
14	Trails at Recreation Park	SY	3000	\$	2.25	\$	6,750.00	\$ 96,750.00	
a b	Prepare sub-base, shape and compact Gravel Base (8" base)	SY	3000	\$	8.00	\$	24,000.00		
C	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	3000	\$	22.00		66,000.00		
				_		_			

5	Landscaping					\$ 36,000.00	
a b	Landscaping Planting Areas (entrances) Loom and Seed Areas (including at demolished bleacher)	EA LS	1	\$ 8,000.00 \$ 20,000.00			"quantity estimated at schematic level
-	Eddin and Seed Areas (including at demonstres presently	1				N I I	
	Site Drainage	LF	600	\$ 28.00	\$ 16,800.00	\$ 47,200.00	*quantity estimated at schematic level
a b	12" HDPE Pipe Catch Basins / Manholes	EA	8	\$ 2,800.00			'quantity estimated at schematic level
c	Nyloplast Drain Structures	EA	5	\$ 1,600.00			quantity estimated at schematic level
	Basketball Court					\$ 41,617,50	
а	Strip and haul topsoil / organics (assume 6" per test pits)	CY	100	\$ 12.00	\$ 1,200,00	41,071,00	
b	Prepare sub-base, shape and compact	SY	470	\$ 2.25	\$ 1,057.50		
C	Gravel Base (8" base)	SY	470	\$ 8.00		4 (1992)	
d	Pavement (1.5" Binder course and 1.5" Wearing Course) Court Surfacing	SY	470 470	\$ 25.00 \$ 10.00			
e	Basketball Hoops	EA	2	\$ 800.00	\$ 1,600.00		
g	10' chain link fence	LF	270	\$ 65.00	\$ 17,550.00		COLUMN TO SERVICE STREET, STRE
	Playgrounds					\$ 110,000.00	
а	Small Play Area	LS	1	\$ 40,000.00	\$ 40,000.00		COLUMN TO STATE OF THE STATE OF
b	Large Play Area	LS	1	\$ 70,000.00	\$ 70,000.00		
						\$ 11,000,00	
а	Seating Areas Benches	LF	20	\$ 400.00	\$ 8,000.00	\$ 11,000,00	
b	Picnic Tables	EA	15	\$ 200,00			C315 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		-					
		-		Subtotal:	TION AREA TOTAL	\$ 3,948,605.23	
				Soft Costs (7%)		\$ 276,402,37	
				10% Contingency		\$ 394,860.52	
				TOTAL		\$ 4,619,868.11	
ADWA	Y			4			
	Entrance Roadway		¥-			\$ 1,179,700.00	
а	Clearcutting	AC	3.2	\$7,000.00	\$ 22,400.00		
C.	Cut & Fill	CY	37100	\$8.00			Excavate/backfill with dozer & haul on site material
d.	Net Cut Excavation & haul	CY	900	\$4.00			Net cut excavation and haul
e	Strip and haul topsoil / organics (assume 6") Gravel Base (12" base)	CY	2600 5300	\$ 12,00 \$ 16.00			
g	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	5300	\$ 25.00	\$ 132,500.00		
h	Signage	LS	1	\$ 3,000.00	\$ 3,000.00		
1	Drainage	EA	12	\$ 2,800.00	\$ 33,600.00		*quantity estimated at schematic level
k	Catch Basins / Manholes	LF	1500	\$ 2,800.00 \$ 50.00			quantity estimated at schematic level
î i	Stormwater Management	LS	1	\$ 80,000.00	\$ 80,000.00		
m	Parking lot Lighting	pole	16	\$ 4,175.00			
0	Walls / Culverts / Wetland Crossings Electric Service to Site	LS	1	\$ 100,000.00 \$ 250,000.00			
			(d)				
				ROADWAY TOTA Subtotal;	L.	\$ 1,179,700.00	
				10% Contingency		\$ 117,970,00	
				TOTAL		\$ 1,297,670,00	
	ALCOHOL: A CONTRACT OF THE PARTY OF THE PART						
SSIVE	RECREATION AREA			1000			
	Parking Area & Access Drive					\$ 179,980.00	
8	Clearcutting	AC	1	\$7,000.00			
b	Strip and haul topsoil / organics (assume 6" per lest pits)	CY	540	\$ 12.00			
d	Gravel Base (8" base) Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	3200 3200	\$ 8.00 \$ 22.00			
e	Signage	LS	.1	\$ 1,500.00		15 - 23 - 24	
f	Drainage	-		0.000 ==	g 40,000 00		Parvaceur and source of the latest and the latest a
g h	Catch Basins / Manholes 12" RCP	EA LF	6 50	\$ 2,800.00 \$ 50.00			'quantity estimated at schematic level 'quantity estimated at schematic level
i	12" HDPE Pipe	LF	300	\$ 50,00	\$ 15,000.00		*quantity estimated at schematic level
	Bioretention Ponds	LS	1	\$ 18,000.00			
k	Parking lot Lighting	pole	4	\$ 4,175.00	\$ 16,700.00		
	Trail Network					\$ 139,250.00	
а	Clearcutting	AC	1.2	\$7,000.00			English A I S ( ) A D I M TO
b	Strip and haul topsoil / organics (assume 6" per test pits)	CY	800	\$ 12.00			
c	Prepare sub-base, shape and compact Gravel Base (8" base)	SY	5000 5000	\$ 2.25 \$ 8.00			
d e	Pavement (1.5" Binder course and 1.5" Wearing Course)	SY	2500	\$ 22.00	\$ 55,000.00		
1	Gravel Surface	SY	2500	\$ 6.00	\$ 15,000.00		
17	Dog Park	4				\$ 143,100.00	
a	Clearcutting	AC	1	\$7,000.00	\$ 7,000.00	170,100.00	EN LA TORRESTA DE LA CONTRACTOR DE LA CO
b	Strip and haul topsoil / organics (assume 6" per test pits)	CY	650	\$ 12,00	\$ 7,800.00		
C	Prepare sub-base, shape and compact	SY	4000	\$ 2.25	\$ 9,000.00		
d	Gravel Base (8" base)	SY	4000 600	\$ 8.00 \$ 22.00			
е	Crushed Stone surface Walkways	Ton	270	\$ 22.00 \$ 30.00			
f	Accessories / stones / benches	LS	1	\$ 10,000.00	\$ 10,000.00		
f			800	\$ 45.00			
f g h	6' chain link fence		1	\$ 20,000.00	\$ 20,000.00		
f g	6 chain link tence Accessories / stones / benches landscaping	LS					
g h		LS		PASSIVE RECRE	ATION AREA TOTA		
f g h		LS		Subtotal:	ATION AREA TOTA	\$ 462,330.00	
f g h		LS		Subtotal: Soft Costs (7%)	ATION AREA TOTA	\$ 462,330,00 \$ 32,363,10	
g h		LS		Subtotal:	ATION AREA TOTA	\$ 462,330.00	

# Enclosure 10 Electrical Consultant Recommendations

### Verne G. Norman Associates, Inc.

Electrical Consultants, Engineers and Designers

Tel: 781-335-4200 Fax: 781-335-5737

January 14, 2013

Mr. John Perry Gale Associates, Inc. 163 Libbey Parkway P.O. Box 890189 Weymouth, MA 02189-0004

Project: Lynnfield Recreation Park

Dear John:

It has been determined that Reading Electric Light Company will be the Utility company which would provide power to the site.

We have discussed the power issue with Peter Price of Reading Municipal Light Department and single phase power is available at the entry to the site and if three phase power is desired than we would need to have Reading Municipal Light Department reconstruct facilities approximate 2/3 miles away from this site to a point near the Sagamore Golf Course on Main street in Lynnfield.

The cost of the primary electric service whether single phase or three phase would be bourne by the Town of Lynnfield.

We have had discussions with Musco Lighting and there is no concern serving their lighting system single phase. This is a very common condition in rural areas.

Ultimately there would be higher wiring cost with a single phase system versus a three phase system but the cost of Reading Municipal Light Department to reconstruct two thirds of a mile of a primary three phase service would be very costly.

The anticipated electric load for the project was based on the following:

Roadway Lighting – 16 poles with 250 watt metal halide lighting fixtures. (Approximately 5.0KW).

Parking Area Lighting – 8 single head poles and 4 double head. 250 watt metal halide lighting fixtures. (Approximately 5.0KW).

Soccer fields – 14 poles consisting of a total of 128 150 watt lighting fixtures. (Approximately 150KW).

Future concession building - Approximately 25.0KW.

Total potential electric load is approximately 185.0 KW.

The site secondary electric service would be 120/240volt single phase, three wire rated at 1000 Amperes.

We would recommend "LED" lighting of the roadway and parking area in lieu of the 250 watt metal halide lighting fixtures. This would result in reduced operating and maintenance cost.

Please feel free to contact our office if you have any questions regarding the aforementioned material.

Very truly yours,

Frederick P. Goff

Frederick P. Goff, P. E.

FG/nh

H:\WPWIN\LYNNFIELD-DEPT. OF PUBLIC WORKS\OUTGOING CORRESPONDENCE\LETTER #1 TO JOHN PERRY.DOCX